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# Probabilistic Assessment of Port Operation Downtimes Under Climate Change 2

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# 12 Abstract

Disruptions in harbor operations have significant implications for local, regional and global 13 economies due to ports strategic role as part of the supply chain. A probabilistic evaluation of port 14 operations considering the influence of climate change is required in order to secure optimal 15 exploitation during their useful life. Here, we propose a hybrid statistic-dynamical framework 16 combining a weather generator and a metamodel. The stochastic generator is based on weather 17 18 types to project climate variability on hourly multivariate dependent climate drivers outside ports. The metamodel efficiently transforms hourly sea conditions from the entrance of the harbor 19 20 towards the inside port adding the advantages of a physical process model. Thousands of hourly 21 synthetic time series based on present climate conditions and future ones were transferred inside 22 the port to perform a probabilistic analysis of port operations. Future forcing conditions were defined adding several sea level rise (SLR) scenarios, sampled from their probability distribution, 23 24 to the synthetic sea level fluctuation time series. Wave amplification due to non-linear interactions between wave and sea level variations and changes in the reflection coefficients inside the port 25 induced by SLR were modelled. Probabilistic future changes of operation downtimes were 26 27 quantified considering the uncertainty associated with the historical forcing conditions outside the port and likely SLR scenarios. The methodology was applied to a specific case study on a regional 28 port located in the north coast of Spain, were port operability due to wave agitation was assessed. 29

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#### 37 **1 Introduction**

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Port infrastructure is strategic for local, regional and global economic growth and development. 38 39 They play a crucial role as transportation hubs and gateways for the vast majority of goods transported around the world, linking local and national supply chains to global markets. 40 Moreover, demands on ports are likely to grow in the light of expected increases in world freight 41 42 volumes, due to shipping efficiency and its smaller carbon footprint compared to other modes of transport [1]. Other economic activities, including industry, tourism and fisheries, also flourish 43 around seaports. Thus, any significant disruption in the logistics of seaports can have significant 44 economic implications [2]. Service disruptions alone can cause considerable economic losses in 45 the order of billions of dollars and may have important second-order consequences, not only for 46 regional economies and the quality of life of those who depend directly on the port's functionality, 47 but also for the operation of global supply-chains [3]. 48

- Due to the type of businesses held around them, seaports are located in one of the most vulnerable areas to climate change impacts, i.e. coastal areas susceptible to sea level rise and increased storm intensity and/or mouths of rivers susceptible to flooding [1]. Despite this, attention to climate-related impacts in ports is relatively recent [4]. The first international benchmark studies consisted of an analysis of the most vulnerable to climate change port cities in 2070 [5] based on population and asset exposure to water levels defined as one hundred year storm surge, and a worldwide survey sent to Port Authorities to detect sectorial perceptions regarding port risks due to climate
- 56 change [1], respectively.

57 The first step in the evaluation of climate change impacts on ports involves reviewing all potential impacts and identifying the main marine variables and the databases available where this 58 information is included [6]. Sea-level rise used to be the only climate-driver considered in the 59 assessment of climate change impacts, as for example, in the methodology proposed to map 60 vulnerability of port assets to sea-level rise relative to their location [2]. Future wave and storm 61 surge conditions are not available from Global Circulation Models (GCMs) for different 62 Representative Concentration Pathway (RCP) scenarios which are the primary tool for 63 investigating the evolution of the climate system over this century. Therefore, a downscaling 64 65 approach is required to obtain such future projections in order to take them into account when assessing the impact of climate change in ports. The assessment of climate change effects on port 66 operability (wave agitation) has been already explored considering changes in waves using various 67 Regional Circulation Models (RCMs) for an A1B scenario [7], or by adding the effect of sea level 68 rise (SLR) in combination with wave changes for one GCM for RCP8.5 [8]. Another example is 69 the simplified approach presented in [9] to assess impacts on port operation due to overtopping at 70 71 the regional scale. This approach consists of a direct statistical weather-typing downscaling of impact indicators (e.g., number of hours per year with overtopping exceeding a certain threshold), 72 integrating changes in storminess including waves, storm surge and sea level rise. One of the 73 74 advantages of this statistical downscaling method is that it allows quantifying the uncertainty associated to different scenarios and climate models (30 GCMs for 2 RCPs were projected), which 75 is not possible if only one or a limited number of GCMs or RCMs are considered. 76

77 Climate drivers for evaluating infrastructure reliability or port operability are defined outside the

port, before local nearshore processes such as breaking, diffraction, or reflection have taken place.

Each hourly set of multivariate marine conditions at the entrance of the harbor has to be propagated

<sup>80</sup> inside the port using a wave model at high spatial resolution. When climate change is assessed to

81 provide useful information for developing effective adaptation strategies, thousands of different

combinations of future forcing variables must be simulated to account for the cascading 82 uncertainty associated with the various scenarios and global/regional models [10]. This multi-scale 83 modelling approach is unaffordable computationally. However, a wide variety of metamodels have 84 85 been proposed to run wave models for large data sets within a reasonable computational time. Metamodels are, in essence, simplified (and hence computationally efficient) representations of 86 computationally intensive models [11]. The traditional approach is to develop a 'look-up table' 87 which involves running the model for a subset of events defined over a regular grid with a coarse 88 89 resolution to limit the number of simulations. Two approaches with a different degree of complexity can be applied to predict the results for additional events: selecting the result of the 90 most similar design point as representative of the new event [8], or using linear interpolation 91 techniques. More sophisticated methods are developed based on the combination of a selection 92 algorithm and radial basis functions [12]. This method has been proved to be quite efficient [13] 93 since it represents the selected input boundary conditions properly and proposes a powerful 94 interpolation technique. Another alternative which doesn't involve numerical simulations consists 95 of applying artificial neural networks to assess port operability [14], but it requires instrumental 96 data outside and inside the port. 97

To assess the safety, serviceability and exploitation of port operations, Spanish Recommendations 98 for Maritime Structures (ROM 0.0-0.1, [15]) propose a Level III Verification Method based on 99 Monte Carlo methods for the probabilistic evaluation of failure modes and operational stoppage 100 modes (downtime) of maritime structures. Modes of failure or operability are determined by non-101 linear interactions of multiple meteo-oceanic dynamics (e.g., astronomical tide, storm surge, 102 waves), climate drivers (waves and storm surge) being statistically dependent due to a common 103 synoptic-scale atmospheric circulation generation. It is therefore necessary to use simulation 104 methodologies that address the dependency among variables. There is a wide range of multivariate 105 statistical models that have been applied to marine conditions. Depending on the type of outputs 106 they provide, models can be divided into two categories: 1) extreme events such as unconditional 107 approaches ([16], [11]); copula methods ([17], [18], [19]); weather-type based models [20] and 2) 108 109 time series using autoregressive models ([21], [22], [23]). The use of Monte-Carlo methods for probabilistic analyses demands a high computational effort to assess infrastructure failure modes 110 or port operability. The process is even more complex if the probabilistic verification is also 111 performed including climate change projections. 112

To our knowledge, only one study has evaluated the effect of climate change in port operability caused by wave agitation due to SLR (three values) and wave changes from one GCM. A metamodel based on the 40 simulations of wave propagation inside the port was applied [8]. Inoperability time was obtained as the sum of the frequencies of occurrence from the wave sets exceeding a fixed threshold. No assessment of port operation downtimes due to wave agitation has been performed using a Monte-Carlo approach, nor including climate change.

119 In this work, we propose an integrated methodology for very long-term probabilistic assessments of port operability due to wave agitation, including the potential effects of climate change. Only 120 port operability due to wave agitation was considered in order to simplify the methodology's 121 description, but the method can be easily extended/used for other applications. The probabilistic 122 verification comprises the use of: 1) a stochastic generator which simulates synthetic multivariate 123 forcing conditions at the entrance of the harbor; and 2) a metamodel to transfer these marine 124 125 conditions inside the port. Synthetic hourly conditions of wave agitation under present and future climate conditions were evaluated to obtain a probabilistic characterization of port operability and 126

127 to assess changes due to climate change. Probabilistic sea level rise (SLR) scenarios were

considered to account for SLR uncertainty in the evaluation of future operation downtimes. The

application of the methodology was particularized to a regional fishing port currently experiencing

130 recurrent downtimes.

The paper is organized as follows: section 2 describes the study area used as a pilot case; section 3 presents the databases required for the application of the methodology and section 4 provides extensive details on the overall methodology which combines a weather generator and a metamodel and describes the impact of climate change on port operations. The application of the methodology to the regional port is presented throughout sections 2–4. Finally, section 5 summarizes and concludes the work.

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# 138 **2 Study area**

The Port of Candás (43° 35, 3' N; 5° 45, 5' W) is located in the region of Asturias (northwest 139 Spain), bordered to the north by the Cantabrian Sea. The current port land area is over 41.150 m<sup>2</sup> 140 with a berthing length of 72 m. The port's main activities are fishing and recreation (see Figure 1). 141 The water depth in the inner harbor varies between 1 and 3 m (Figure 2). The main breakwater has 142 a trapezoidal cross-section consisting of an outer layer of 23 ton concrete cubes, a secondary layer 143 with 2-3 tons of gravel, a 50-1000 kg rubble layer and a core. A concrete crown wall lies on top 144 of the rubble mound breakwater with a crest level of 11.50 m. The geometry and materials of the 145 different natural and artificial structures of the port's inner boundaries cause changes in wave 146 reflection along these boundaries at different water levels. For this case study these variations were 147 included in the agitation model by using different reflection coefficients along the berths and docks 148 149 for the four sea levels considered (see Figure 2). Specifically, the following reflection coefficients were considered, according to the typology they represent: dissipative beach (Kr=0.15), reflecting 150 beach (Kr=0.20), rubble-mound breakwater (Kr=0.40), cliff (Kr=0.60) and vertical wharf 151 (Kr=0.9). For low and mean tide reflection coefficients were kept constant. 152



Figure 1. Location of the Port of Candás in northern Spain.



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Figure 2. Upper figure: Bathymetry of the study area (depth in meters). Lower figures: Reflection 157

- coefficients adopted along the port boundaries under different sea levels: low and mean tide, high 158 tide and high tide + SLR. Kr=0.15 for dissipative beach, Kr=0.20 for reflecting beach, Kr=0.40 159
- for rubble-mound breakwater, Kr=0.60 for cliff and Kr=0.9 for vertical wharf. 160
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#### 162 **3** Databases

Sea level pressure fields of the Climate Forecast System Reanalysis (CFSR and CFSRv2; [24]) 163

were used to define the predictor of the statistical models explained in section 4. The temporal 164

coverage spanned from 1979 to 2013, with an hourly temporal resolution and a 0.5° spatial 165

- resolution. 166
- The historical wave information used was the high resolution coastal wave database Downscaled 167
- Ocean Waves (DOW, [25]), with a low resolution mesh of 0.01°x0.008° and several nested meshes 168
- reaching a maximum resolution of 200 m. This database was generated using a hybrid downscaling 169

170 methodology which combines statistical techniques and dynamical simulations. The Global Ocean

Waves database (GOW, [26]) was used at the regional scale as wave forcing to generate the coastal 171

wave reanalysis. The SeaWind database, generated by performing a dynamical downscaling of the 172

173 NCEP/NCAR wind reanalysis at a spatial scale of 30 km [27], was used as wind forcing. The

results of this hybrid downscaling provided the following hourly sea state parameters from 1948 174

to 2014: significant wave height ( $H_s$ ), mean period ( $T_m$ ), peak period ( $T_n$ ) and wave direction ( $\theta$ ). 175

The 62-year (1948-2014) high-resolution hindcast of the meteorological sea level component 176 (storm surge, SS) (GOS 1.1; [28]) was used to determine historical storm surge data. The GOS 1.1 177

database was developed for Southern Europe using the Regional Ocean Model System (ROMS) 178

with a horizontal resolution of  $1/8^{\circ}(\sim 14 \text{ km})$ . 179

180 The astronomical tide (AT) was reconstructed on an hourly basis at a spatial resolution of 0.25°,

using harmonic analyses on the outcomes of the global model of ocean tides (TPXO7.2) that 181

- assimilates data from TOPEX/Poseidon missions and tidal gauges for the common period of waves 182
- and storm surge. 183

The regional SLR by 2100 for RCP8.5 scenarios was extracted from global projections of regional 184 mean sea level values obtained by [29] using a dynamical modeling approach that incorporates 185 regional contributions of land ice, groundwater depletion and glacial isostatic adjustment, 186 including gravitational effects due to mass redistribution. 187

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#### 189 4 Methodology and results

190 The methodology described in Figure 3 is composed of two main parts:

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  - A weather generator to derive hourly multivariate marine conditions outside the port.

192 • A metamodel to transfer hourly marine conditions outside the port as generated in the previous step to the inner harbor, in order to obtain wave agitation. 193

The definition of the stochastic generator requires historical information of the forcing conditions 194 outside the port. The climate emulator based on weather patterns for modelling daily multivariate 195 events [20] was extended to simulate hourly waves and storm surges at the entrance of the port. 196 The model is based on a predictor-to-predictand synoptic regression-guided classification [9], 197 grouping marine conditions according to similar generating meteorological processes, called 198 weather types (WTs). This method ensures that the predictand within each WT is independent and 199 identically distributed for the applicability of Gaussian copulas to model the dependence between 200 variables. Besides, the method captures climate's non-stationary characteristics based on the 201 variability of WTs over time. A Monte Carlo approximation is applied to stochastically simulate 202 large samples of hourly conditions at the entrance of the harbor. 203

204 For the second step, a metamodel based on a hybrid downscaling methodology (a combination of dynamical and statistical downscaling) developed to generate high resolution nearshore wave 205 reanalysis databases [25] was adopted. Specifically, a number of representative sea states was 206 propagated using a model solving the elliptic mild slope (MSP, [30]) and the time series of 207

- nearshore wave parameters were reconstructed by means of an interpolation technique. The way in which the number of simulations was selected from the synthetic data ensured the coverage of the new multivariate space of climate drivers. The probabilistic assessment of current port operability due to wave agitation was obtained by reconstructing the significant wave height inside
- operability due to wave agitation was obtained by reconstructing the significant wave height inside the port for each simulated hourly condition at the entrance of the harbor for the present climate.

213 To assess climate change impacts on port agitation, climate change can be introduced in the

- weather generator by means of future WT probabilities that can be reflected as changes in waves
- and storm surges and SLR added to the sea level time series. The metamodel has to be updated to
- take into account climate change in those cases selected to be modelled as well as the effect of
- SLR on the reflection coefficients to be used in the wave agitation model.



Figure 3. Probabilistic methodology which combines a weather generator and a metamodel to assess port operability due to wave agitation under present and future conditions.

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Figure 4 shows the time series for years 2013 and 2014 and the distribution of  $H_s$ - $\theta$  and  $T_p$ - $\theta$  of the forcing conditions occurring outside the port, obtained from the databases described in section 2. Forcing conditions outside the port were defined at about 6.0 m depth. Wave climate at this location has suffered an intense refraction due to the protection effect of Cape Peñas (Figure 1) resulting in wave energy concentration in the N-E sector. The maximum significant wave height is limited to 4.5 m while peak periods reach values of 20 s which can be combined with storm surges of almost 0.5 m and high spring tides over 2.0 m.



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Figure 4.  $H_s$ ,  $T_p$ , SS and AT values for two years within the time series ) at the entrance of the harbor (left panels).  $H_s$  and  $T_p$  roses (right panels).

4.1 Weather generator

234 A weather-type framework was used to model the nonstationary behavior of the local multivariate predictand ( $H_s$ ,  $T_m$ ,  $T_p$ ,  $\theta$  and SS) related with large-scale predictors (sea level pressure, SLP). The 235 daily predictor was classified into a discrete number of weather patterns (WTs) according to their 236 synoptic similarity. Hourly multivariate events were modelled using a marginal distribution for 237 each predictand variable and a Gaussian copula within each WT. The stochastic generator follows 238 similar steps as the one developed by [20] for multivariate extremes, except in this case the 239 extremal index is not required. The five steps involved in this model are: 1) To collect and pre-240 process historical data of the predictor (SLP) and predictands ( $H_s$ ,  $T_m$ ,  $T_p$ ,  $\theta$  and SS). 2) Define 241 WTs using a semi-guided classification [31]. 3) Fit a stationary model (e.g. Lognormal, 242 Generalized Extreme Value) to each variable of the multivariate predictand ( $H_s$ ,  $T_m$ , SS outside the 243 port) associated with each WT. 4) Model the dependence between predictand variables within each 244 weather type using a Gaussian copula. 5) Generate synthetic multivariate hourly conditions taking 245 into account the monthly WT probability and dependence structure associated with each WT. 246

The spatial domain of the predictor should cover the oceanic region responsible for generating waves arriving at each location of interest. The temporal coverage (recent history) should account for wave travel time from generation to target location. Based on previous works, the semi-

supervised WTs of the grid node from the global collection of WTs at a 1.0°×1.0° resolution 250 generated to obtain global wave projections [9] at a location closest to the port of study, was used 251 to develop the weather generator (steps 1 and 2 in this section). The predictor definition (spatial 252 domain and temporal coverage) corresponded to the subdomain covering the North Atlantic Ocean 253 (from an ocean division based on a global wave genesis characterization). The predictor was 254 defined as the 3-daily mean SLP and 3-daily mean SLPG (squared SLP gradients), calculated daily 255 throughout the historical time period. More details regarding this characterization and WT 256 collection can be found in [9]. A regression guided classification was applied to a combination of 257 the weighted predictor and predictand estimations from a regression model linking the SLP fields 258 with local marine conditions. The level of influence of the wave and storm surge data was 259 controlled by a simple weighting factor which balances the loss/gain of predictor/predictand 260 representativeness. A factor equal to 0.6 was implemented based on previous sensitivity analyses. 261 A better grouping of the predictand was obtained due to a stronger relation of the WTs with local 262 marine climate conditions. 263

The long-term marginal distributions (step 3) of hourly  $H_s$ ,  $T_m$ , and SS outside the port within each WT were fitted to a generalized extreme value (GEV) distribution, a Lognormal distribution or Unified Distribution Model [32], obtaining the best fit of the central regime with a GEV. The empirical distribution was used for the wave direction variable. A heteroscedastic model between  $T_p$  and  $T_m$  was fitted within each WT. Tp was considered to be normally distributed with parameters mean and variance being a function of  $T_m$  (polynomials with unknown degree). A Gaussian Copula was used to model the dependence between  $H_s$ ,  $T_m$ , SS and  $\theta$  (step 4).

- 271 The Monte Carlo sampling procedure used to generate synthetic marine conditions (step 5) requires the following phases: i) Sample a daily WT from a Generalized Bernoulli distribution due 272 to the categorical choice of one of the N=100 WTs. ii) Randomly generate 24 hourly synthetic  $H_s$ , 273  $T_m$ ,  $\boldsymbol{\theta}$  and SS using the Gaussian copula and the marginal fits associated with the daily simulated 274 WT; iii) Sample 24 hourly  $T_p$  from the heteroscedastic model between  $T_p$  and  $T_m$  associated with 275 the daily WT; iv) Independently sample 24 hourly values of astronomical tide from its monthly 276 empirical distribution. The process is repeated until a synthetic 90-year time series of hourly 277 multivariate forcing marine conditions is obtained. 278
- One thousand, 90-year long, new time series of  $H_s$ ,  $T_m$ ,  $T_p$ ,  $\theta$ , SS and AT were simulated with the
- previously fitted emulator. Each series was generated with a different set of parameters, randomly taken from the parameter sample obtained considering a Gaussian distribution. Scatter plots of the
- five sea-storm variables are shown in Figure 5. The large multivariate sample of hourly forcing 282 conditions captures the characteristics of dependencies among variables. Wave breaking and wave 283 steepness limit the maximum simulated wave height. Maximum simulated wave period was 284 limited to 25 s. The effect of the imposed physical limitations of wave slope can be observed in 285 the correct reproduction of the relation between wave heights and small wave periods. Figure 6 286 shows the joint probability density functions of  $(H_s, T_p)$ ,  $(H_s, \theta)$ ,  $(H_s, SS)$  and  $(T_m, T_p)$  obtained 287 from the historical series (blue lines) and from the simulated series (dashed lines). The simulated 288 289 series are able to reproduce the main features of the original bivariate distributions. They fail in representing some details of the distributions, as the clear dependence between wave heights 290 around 1.0 m and low peak periods. 291
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Regarding future synthetic time series (e.g. in the period 2010-2100), climate change can be introduced taking into account changes in storminess by means of future WT probabilities from GCMs and the increase in the mean sea level. Robust multi-model ensemble projections at high spatial resolutions (0.01°x0.008° using DOW at the reference database) measured over the whole

century (2010–2099) were estimated along the northern coast of Spain [33]. Future wave and storm

surge projections were statistically downscaled using a weather-type approach [34] for the same 311 40 GCMs as in regional wave projections made in Europe [35]. The statistical relationship was 312 established as in the first steps of the weather generator. In this case, however, the empirical 313 314 probability distribution of each sea state parameter (e.g., significant wave height) associated with each WT was calculated. The distribution of this variable for a certain time period can be estimated 315 as the sum of the probability of each WT during that period multiplied by the corresponding 316 empirical distribution. Different statistics (e.g., mean, 95th percentile) can be derived from the 317 estimated distribution. One of the advantages of this statistical downscaling methodology is that 318 the scale representativeness of the projections depends on the underlying historical wave databases 319 used as a reference [9]. Figure 7 shows the multimodel ensemble projections of the annual mean 320 and the 95<sup>th</sup> percentile of the significant wave height, the mean period and the 95<sup>th</sup> percentile of 321 storm surge in the area surrounding the port for the period 2070-2099 compared with the 1979-322 2010 period under the RCP8.5 scenario. Box plots illustrated the uncertainty inherent in future 323 changes obtained from the 40 GCMs. The outcomes reveal slight decreases in surge and wave 324 height and period. These changes are assumed to be negligible compared to the effect of SLR in 325 wave propagation inside the port. Indeed, the decreasing waves and storm surge resulting from 326







**Figure 7.** Regional multimodel projections (RCP8.5, 2070–2099 with respect to 1979–2005) for the mean and the 95<sup>th</sup> percentile of wave significant wave height, mean wave period and the 95<sup>th</sup> percentile of storm surge along the coastline surrounding the study port.

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333 Following the approach proposed by [36] to account for the SLR uncertainty in the assessment of flooding risk, a lognormal distribution was fitted with the mean and standard deviation of the 334 335 regional projections produced by [29] for the RCP8.5 scenarios in 2100 (i.e.,  $0.63 \pm 0.20$  m at the study area). The lognormal distribution is considered the most likely distribution representing 336 future SLR [37], although increased rates of ice sheet loss were not included in this study. The 337 deciles from fitted lognormal distributions split the SLR data set from each horizon year into ten 338 339 equally probable parts, the 2100 deciles being: 0.377 m; 0.454 m; 0.507 m; 0.553 m; 0.599 m; 0.646 m; 0.699 m; 0.763 m; 0.852 m; and 1.025 m, respectively. Ten curves were derived from 340

local RCP8.5 SLR values in 2025, 2050 and 2100 using a second order polynomial function in
order to adopt the shape of those provided by the IPCC [38]. Hourly SLR time series (2010-2100)
derived from these curves were added to the synthetic sea level time series (defined as the sum of
storm surge and astronomical tide) to define future forcing conditions of port agitation.

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## 3464.2 Metamodel

The steps followed to define the metamodel used to transform all the synthetic forcing conditions outside the port were: 1) selection of a limited number of cases comprising the most representative scenarios of wave and sea level fluctuations (storm-surge, astronomical tide and sea level rise) outside the port; 2) a wave agitation strategy to propagate the selected sea states from the entrance of the bay towards the inner harbor zone; 3) reconstruction of the time series of significant wave heights inside the port.

A subset of sea states (M=500) representative of marine conditions outside the port was selected 353 using the maximum dissimilarity algorithm (MDA, [12]). The MDA identifies a subset comprising 354 the most dissimilar data in a database. The selection starts by initializing the subset through the 355 transference of one vector from the data sample. The remaining elements are selected iteratively, 356 transferring the most dissimilar one from the remaining data in the database to the subset. Figure 357 8 shows the distribution of the selected subset from the MDA over the full multivariate parameter 358 space ( $H_s$ ,  $T_p$ ,  $\theta$  and sea level) covered by the Monte Carlo realizations. The multivariate subset is 359 distributed evenly across the space covering the potential combinations between the four variables 360 with some points selected in the outline of the data space, contributing to an accurate reconstruction 361 of wave agitation conditions inside the port using the proposed metamodel. 362



Figure 8. Scatter plots of simulated data (grey dots) and the selected cases using the MDA (red dots in).

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The MSP numerical model [30] was used for wave agitation simulations. This model is able to 367 solvewave propagation towards and into the harbor, taking into account the refraction, diffraction, 368 wave breaking and partial reflection imposed by natural and artificial structures (quays, basins, 369 breakwaters, etc.) and real bathymetry contours. The model provides (2DH) significant wave maps 370 along the whole numerical domain. A complete spectral sea-state propagation strategy [39] based 371 372 on the invocation of a pre-calculated monochromatic wave catalogue was applied to noticeably reduce the CPU-effort to propagate real wave spectra towards any inner control point. This 373 technique is based on a three-step method: 374

1. The selection of N monochromatic wave conditions (the combination of periods T and directions  $\theta$ ) by collapsing the 4D-hypermatrix [frequency, direction, energy, time] for the whole wave hindcast used, into a single resulting matrix representing the historical energy packs available in the study zone (for a typical 35 frequency x 72 direction spectrum matrix, and taking into account real/theoretical frequency and direction spreading factors for each hour). N can adopt values from 30% to 60% of the total matrix size used, depending on the geographical location of
 the harbor/ outer wave climate.

- The numerical propagation of each N monochromatic wave (using a constant wave height
   H, because of the linear nature of the model used) and for the different sea levels considered.
- 384 3. The aggregation of any spectrum by adding all the individual energy packs that define it.
- 385 For this study additional considerations were established:
- 4. Four water levels were used (total Nx4 monochromatic cases) (three to cover the astronomical tide range and one as expected upper SLR).
- 5. Changes in reflection coefficients in the model's setup (as described in the study area section) due to SLR.

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391 This technique, besides achieving a radical CPU-time reduction, enables to rapidly include any future scenario needed or sensitivity analysis required, as well as changes in one or many spectrum 392 variables due to climate change (energy, frequency, direction and its frequency-directional 393 394 spreading). On the other hand, this technique could over-predict wave-shoaling effects, especially for shallow bathymetry zones. Thus, it should be used with caution if non-linear wave-wave 395 interactions are expected in the study zone, especially for wave breaking related processes and 396 397 shoaling. This drawback is minimized for open harbors, with (in general) quasi-constant/ mild bathymetry configurations within the basins and outer zones, as shown in [39]. Figure 9 shows an 398 example of a wave agitation map inside the port for conditions outside the port defined by  $H_s=7.2$ 399 400 m;  $T_p=15.8$  s;  $\theta = 54.5^{\circ}$  and sea level=3.63 m.



**Figure 9.** Wave agitation map for the following marine conditions outside the port:  $H_s$ =7.2 m;  $T_p$ =15.8 s;  $\theta$  =54.5° and sea level=3.63 m.

The significant wave height time series inside the port are reconstructed using the 404 multidimensional interpolation technique of radial basis functions (RBF, [40]). The RBFs enable 405 a statistical relationship to be defined between the marine parameters characterizing the forcing 406 conditions and the wave height inside the port from the results of the selected cases. The RBF 407 interpolation method defines the function to be approximated by means of a weighted sum of 408 radially symmetric basic functions located at the data points where the results are available. A 409 more detailed description of these statistical tools implemented in the proposed hybrid 410 methodology can be found in [12]. 411

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#### 413 4.3 Results

The synthetic historical time series of marine conditions (waves, storm surge, and astronomical tide) outside the port were transferred inside the port using the corresponding RBF. The synthetic time series outside the port was transformed to the future period 2010-2099 adding the corresponding SLR to each hourly sea level. Their corresponding wave height inside the port was reconstructed applying the RBF.

The annual operability or the hours of non-operability are some of the basic port design criteria stipulated by national and/or international standards (such as ROM or PIANC). In this example, hours of non-operability were calculated from each time series as the hours exceeding a certain threshold of  $H_s$  inside the port. Here, a threshold of 0.4 m was applied, as suggested in the Spanish Recommendations for Maritime Structures for Fishing Ports (ROM 3.1-99, [41]).

Figure 10 shows the historical and future empirical cumulative distributions of significant wave 424 height, Hs, inside the port in Areas 1 and 2 from the one thousand synthetic time series. Fifty-year 425 long time series of forcing conditions were considered in the assessment of the port's downtimes 426 since the useful life of the Port of Candás is established in 50 years. The future distribution was 427 428 based on the thousand synthetic future hourly time series from 2050-2099 obtained for the ten SLR scenarios sampled from a lognormal distribution of the RCP8.5 SLR projections. The future 429 empirical distributions for the ten SLR scenarios were represented in a yellow-red scale 430 corresponding to the lowest-highest decile, respectively. The probability of a significant wave 431 height lower than 0.4 m (non-operability threshold for fishing ports) is lower the higher the SLR 432 (see the zoomed image of the empirical cumulative distribution between 0.35 to 0.45 m in Figure 433 10). 434



Figure 10. Historical empirical cumulative distribution (in blue) and future empirical cumulative distributions for the ten SLR scenarios (in yellow-red scale) of significant wave height inside the port in Area 1 and Area 2.

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Hours of non-operability were calculated from the probability (p) obtained for a threshold of 0.4 440 m as  $(1-p) \times 365 \times 24$ . The probabilistic distributions of non-operability hours at present (blue) and 441 future (2050-2099, in red) climate conditions are shown in Figure 11. Future distributions of non-442 operability for each RCP8.5 SLR scenario are displayed (dashed lines in the yellow-red color 443 scale) with the ensemble mean future probabilistic distribution of non-operability (in red). The 444 ensemble mean distribution was obtained by adding up the distribution for each of the ten SLR 445 scenarios multiplied by 0.1 (the ten SLR scenarios are sampled with an equal probability). It can 446 be noted that hours of non-operability do increase from present to future conditions for both areas. 447

The probabilistic distribution of non-operability hours under current climate conditions represents the uncertainty associated with the historical forcing conditions outside the port. The ensemble mean future distribution integrates the uncertainty associated with the RCP8.5 SLR scenarios and the uncertainty due to the forcing conditions outside the port (distributions of non-operability hours of ten future RCP8.5 SLR scenarios). Besides, higher non-operability hours, as well as a higher uncertainty, are expected for higher SLR scenarios, as can be observed in wider probabilistic distributions of non-operability hours the higher the SLR decile (see Figure 11).

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**Figure 11.** Probabilistic distributions of non-operability hours due to wave agitation in Areas 1 and 2 inside the port in current (in blue) and future (2050-2099) climate conditions in each SLR scenario (in yellow-red scale) and the future ensemble mean distribution of non-operability hours (thick red line).

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464 One additional way to summarize and compare the results obtained is displayed in Table 1. Each of the present and future distributions of non-operability hours is fitted to a lognormal distribution. 465 466 The mean, standard deviation and coefficient of variation are calculated and shown in the table. In both Areas 1 and 2, the future mean values for non-operability hours are drastically increased (from 467 198.90 to 332.13 in Area 1 and from 313.62 to 475.14 in Area 2) due to a mean SLR of 0.257 m 468 by 2050 and 0.634 m by 2100. The latter increase is due to non-linear interactions between waves 469 and sea level, and changes in the reflection coefficients associated to SLR. Regarding the 470 nondimensional coefficient of variation, the uncertainty associated with non-operability hours 471 increases from about 1.3 % (0.0132 in Area 1 and 0.0129 in Area 2) under current conditions, to 472 around 10 % (0.1163 in Area 1 and 0.1044 in Area 2) in future ones. These results differ from the 473 future SLR coefficient of variation (26.7% by 2050 and 31.1% by 2100), indicating that the 474 magnitude of the SLR uncertainties are reflected to a lower degree in the magnitude of the 475 uncertainty of non-operability hours. 476

	RCP8.5 SLR (m)		Non-operability (hours)				
			Area 1		Area 2		
	2050	2100	Present	Future (2050-2100)	Present	Future (2050-2100)	
Mean	0.257	0.634	198.90	332.13	313.62	475.14	
Std	0.069	0.197	2.625	38.61	4.057	49.604	
CV (std/mean)	0.267	0.311	0.0132	0.1163	0.0129	0.1044	

Table 1: Mean, Standard Deviation and Coefficient of Variation of the lognormal distribution of 477

2050 SLR and 2100 SLR predictions and the lognormal distribution of the non-operability hours 478

for the present and future period in Areas 1 and 2. 479

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481 Non-operability hours during the useful life of the infrastructure/port taking into account climate change was calculated along the 21st century adding the evolution of the SLR to the hourly time 482 series of sea level. In the previous analysis of the impact of climate change in the port's operability, 483 future non-operability hours were calculated for a useful life of 50 years, from 2050 to 2100, to 484 obtain more significant changes. However, the assessment of port operability should be adjusted 485 to the projected useful life of the infrastructure, as of its construction. Figure 12 shows the 486 487 interannual variability of the ensemble's mean probability of non-operability hours from 2010 to 2099 in Areas 1 and 2 based on the ten SLR scenarios. First, the empirical distribution of non-488 operability hours was calculated on a yearly basis for each of the 10 RCP8.5 SLR scenarios 489 considered. Mean sea level rise rates were determined fitting a second order polynomial to the 490 deciles from the local SLR lognormal distribution in 2025, 2050 and 2100. Afterwards, the 491 ensemble mean distribution of non-operability hours was calculated every year. An average 492 493 moving mean of ten years was applied. A linear trend of the mean hours of non-operability along the 21st century can be observed in Figure 12 (e.g., downtime increases from 320 hours in 2010 494 to 510 hours in 2100). The dispersion of the empirical density distribution rises along the 21st 495 century due to a broader uncertainty of the SLR scenarios as the horizon increases. At the 496 beginning of the 21<sup>st</sup> century, the SLR distribution spread was limited which is reflected in a 497 narrow ensemble mean distribution of non-operability hours (i.e., high probability centered in the 498 mean value). However, the SLR distribution broadened along the 21st century, increasing the 499 ensemble mean distribution of hours of non-operability (e.g., downtime hours vary from 290 to 400 500 hours in 2010 and from around 400 to 800 in 2100). Changes in non-operability hour values are 501 more significant after 2050 due to a more pronounced acceleration of SLR as of the second half of 502 503 the 21st century.



Figure 12. Interannual ensemble mean probability of non-operability hours from 2010 to 2099 in
 Area 1 and Area2 taking into account the increase in SLR uncertainty along the 21<sup>st</sup> century.

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#### 508 **5 Summary and conclusions**

A hybrid statistical-dynamical framework was developed with two main purposes: 1) to provide a probabilistic evaluation of port operability to assess a minimum level of downtime of the port; 2) to introduce climate change in the assessment of port operability during its useful life.

The methodology is strongly dependent on the multivariate nature of climate drivers of wave agitation such as the combination of waves and sea levels and the availability of these forcings outside the port. Therefore, the following requirements should be met: 1) the use of a stochastic generator to model the dependence between multivariate conditions; 2) the application of a numerical modelling approach to propagate wave offshore conditions inside the port.

Hence, the methodology includes: 1) A weather generator based on WTs to take into account future climate variability through WT probability changes linked to changes in climate drivers (waves and storm surges); 2) A metamodel based on a catalog of wave propagations and a multidimensional non-linear interpolation to reconstruct hourly significant wave height time series inside the port with an accuracy similar to that of the numerical simulations.

The case study was focused on port operability due to wave agitation. The methodology allows to 522 transfer thousands of synthetic time series of present and future climate conditions inside the port 523 in order to carry out a probabilistic analysis of port operability. Future changes in non-operability 524 525 are expressed including both uncertainties associated with marine conditions outside the port and SLR. Climate induced changes in waves and storm surge are considered to be negligible due to 526 the projections obtained in the study area. Uncertainty of forcing conditions outside the port was 527 quantified through the use of a weather generator that allows to generate synthetic time series. 528 SLR uncertainty was introduced equally by sampling its probability distribution in several 529 horizons, while hourly SLR time series were added to the synthetic sea level fluctuations to define 530 the future forcing conditions outside the port. SLR uncertainty was integrated in the future non-531 532 operability evaluation joining the contribution of each sampled SLR scenario with its corresponding probability. 533

Obtaining the future distribution of non-operability hours allows calculating the future probability associated with the non-operability exceedance hours threshold established in port design recommendations (i.e. ROM 3.1-99, [41]) during their useful life. The proposed hybrid methodology produces this very useful and relevant outcome to define a specific acceptable operability risk and can be used as a design criterion in new coastal infrastructure or for climate change adaptation plans.

Although for this specific pilot case, climate induced changes on waves and storm surges, have been neglected due to their small values, non-linear feedbacks induced by SLR that may produce an amplification of wave conditions in shallow waters [42] have been introduced in the wave agitation model. Future hourly sea conditions are transformed from the harbor's entrance to inside the port considering the non-linearities between tides, surges, waves and SLR. Changes in the reflection coefficient inside the port due to changes in sea level have also been implemented in the wave agitation simulation.

The proposed methodology presents several limitations. The synthetic marine conditions are generated without modelling time structure dependence, which would allow performing an analysis of non-operability's persistence. Besides, this version of the climate emulator is not useful for the analysis of extreme conditions. Synthetic extreme events are not time independent and their frequency could be overestimated. Nevertheless, our objective was focused on port operability which should not be conditioned by extreme events.

The methodology presented can be extended to further applications such as coastal infrastructure reliability or operability for other functional parameters or marine operations by tailoring the weather generator and selecting the most appropriate numerical model.

556

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# Probabilistic Assessment of Port Operation Downtimes Under Climate Change 2

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#### 14 Abstract

Disruptions in harbor operations have significant implications for the-local, regional and global 15 economiesy due to ports<sup>2</sup> strategic role as part of the supply chain. A probabilistic evaluation of 16 port operations taking into account considering the influence of climate change -is required in order 17 to secure optimal exploitation during their ports' useful life. Here, we propose a hybrid statistical-18 dynamical framework is proposed combining a weather generator and a metamodel. The stochastic 19 generator is based on weather types to project climate variability on to-hourly multivariate 20 dependent climate drivers outside the ports. The metamodel efficiently transforms hourly sea 21 conditions from the entrance of the harbor towards the inside the port addinglevering the 22 advantages of a physical process model. Thousands of hourly synthetic time series based on at 23 present climate conditions and in the future ones wereare transferred inside the port to perform a 24 probabilistic analysis of port operations. Future forcing conditions wereare defined adding several 25 sea level rise (SLR) scenarios, sampled from theirits probability distribution, to the synthetic sea 26 level fluctuations time series. Wave amplification due to non-linear interactions between waves 27 and sea level variationss and changes in the reflection coefficients inside the port induced by SLR 28 wereare modelled. Probabilistic future changes of operation downtimes wereare quantified 29 considering with the uncertainty associated withto the historical forcing conditions outside the port 30 and the likely SLRsea level rise scenarios. The application of the methodology was applied to a 31 specific a case study on of a regional port located ion the north coast of Spain, were is described 32 for port operability due to wave agitation was assessed. 33 34 35

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## 41 **1 Introduction**

Port infrastructuress isare are strategic infrastructures for for local, regional and global economic 42 growth and development. They play a crucial role as transportation hubs and gateways for the vast 43 of majority of goods transported around the world, linking local and national supply chains to 44 global markets. Moreover, demands on ports are likely to grow in the light of expected increases 45 46 in world freight volumes, due to shipping shipping's efficiencies efficiency of shipping and its smaller carbon footprint compared relative to other modes of transport when dealing with an 47 expected increase in world freight volumes [1]. Other economic activities, including industry, 48 tourism and fisheries, also flourish around seaports. Thus, Aany significant disruption in the 49 logistics of seaports can have significant economic implications for the economy [2]. Service 50 disruptions alone can cause considerable total economic losses in the order of billions of dollars 51 52 and may have important second-order consequences, not only for the regional economiesy and the quality of life of those who depend directly on the port's functionality, but also for the operation 53 of global supply-chains [3]. 54

Due to the type By the nature of their businesses held around them, seaports are located in one of 55 the most vulnerable areas to climate change impacts, i.e.: in-coastal areas susceptible to sea level 56 rise and increased storm intensity and/or at-mouths of rivers susceptible to flooding [1]. The 57 58 aDespite this, attention to climate-related impacts in ports is relatively recent [4]. The first international benchmark studies consisted of are at international scale: an analysis of the most 59 60 vulnerable to climate change port cities to climate change in 2070 [5] based on population and asset exposure of population and assets to a water levels defined as one hundred year storm surge. 61 and a worldwide survey sent to ofof Port Authorities to detect sectorial's risk perceptions 62 regardingabout port risks due to climate change on ports [1], respectively. 63 The first main first step in the evaluation of climate change impacts on ports involves reviewing 64

allthe potential impacts of climate change on ports, and identifying the main marine variables and
 the available databases available where this information is included to obtain and process the
 relevant information [6]. Sea-level rise used to be the only climate-driver to be considered in the

- assessment of climate change impacts, as for example, in the methodology proposed to map
- 69 vulnerability of port assets to sea-level rise relative to their location [2]. Future wave and storm
- surge conditions are not available from Global Circulation Models (GCMs) for different
   Representative Concentration Pathways (RCPs) scenarios which are the primary tools for
- 72 investigating the evolution of the climate system over this century. <u>Therefore</u>, <u>Aa</u> downscaling
- 73 approach is required to obtain <u>such</u> future projections of waves and storm surge in order to <u>take</u>
- 74 <u>them into account when assessing introduce these changes in the assessment of</u> the impact of 75 climate change in ports. The assessment of climate change effects on port operability (wave
- climate change in ports. The assessment of climate change effects on port operability (wave agitation) has been already explored considering changes in waves using various Regional
- Circulation Models (RCMs) for an A1B scenario  $[7]_{a}$  or by adding the effect of sea level rise (SLR)
- in combination withof wave changes for one GCM for RCP8.5 [8]. Another example is the
- <sup>79</sup> simplified approach presented in [9] to assess<del>, at regional scale,</del> impacts <u>on port operation</u> due to
- 80 overtopping at the -regional scaleon port operation. This approach consists of a direct statistical
- 81 weather-typing downscaling of impact indicators (e.g., number of hours per year with overtopping
- exceeding a certain threshold), integrating changes in storminess including waves, storm surge and sea level rise. One of the advantages of this statistical downscaling method is that it allows to

- quantifying the uncertainty <u>associated to coming from dd</u>ifferent scenarios and climate models (30
- 85 GCMs for 2 RCPs were projected), whichat is not possible if only one or a limited number of
- 86 GCMs or RCMs are considered.

Climate drivers for evaluating infrastructure reliability or port operability are defined outside the 87 port, before local nearshore processes such as breaking, diffraction, or reflection have taken place. 88 89 Each hourly set of multivariate marine conditions at the entrance of the harbor has to be propagated inside the port using a wave model at high spatial resolution. When In the case of assessing climate 90 change is assessed to provide useful information for developing effective adaptation strategies, 91 thousands of different combinations of future forcing variables must be simulated to account for 92 93 the cascadinge uncertainty associated with the various scenarios and global/regional models [10]. This multi-scale modelling approach is unaffordable computationally. However, Aa wide variety 94 95 of metamodels have been are proposed to run wave models for large data sets within a reasonable computational time. Metamodels are, in essence, simplified (and hence computationally efficient) 96 representations of computationally intensive models [11]. The traditional approach is to 97 developing a 'look-up table' which involves running the model for a subset of events defined over 98 a regular grid with a coarse resolution to limit the number of simulations. Two approaches with a 99 different degree of complexity can be applied to predict the results for additional events: selecting 100 the result of the most similar design point as representative of the new event [8], or by using linear 101 102 interpolation techniques. More sophisticated methods are developed based on the combination of a selection algorithm and radial basis functions [12]. This method has been proved to be quite 103 efficient [13] since it represents the selected input boundary conditions properly and proposes a 104 due to the proper representation of the selected input boundary conditions and the powerful 105 interpolation technique. AnOother alternative which doesn't involve without numerical 106 simulations consists of applying artificial neural networks to assess port operability [14], but itthey 107 requires -instrumental data outside and inside the port. 108

TFurthermore, to assess the safety, serviceability and exploitation of port operations, the Spanish 109 110 Recommendations for Maritime Structures (ROM 0.0-0.1, [15]) proposes a Level III Verification Method based on Monte Carlo methods, for the probabilistic evaluation of failure modes and 111 operational stoppage modes (downtime) of maritime structures. Modes of failure or operability are 112 113 determined by non-linear interactions of multiple meteo-oceanic dynamics (e.g., astronomical tide, 114 storm surge, waves), being-climate drivers (waves and storm surge) being statistically dependent due to a common synoptic-scale atmospheric circulation generation. It is therefore necessary to 115 116 use simulation methodologies that address the dependency between/among variables. There is a wide range of multivariate statistical models that have been applied to marine conditions. 117 118 Depending on the type of outputs they provide, models can be divided into two categories: 1) 119 extreme events such as unconditional approaches ([16], [11]); copula methods ([17], [18], [19]); weather-type based models [20] and 2) time series using autoregressive models ([21], [22], [23]). 120 The use of a Monte-Carlo methods for probabilistic analyse is demands a high computational effort 121 122 to assess infrastructure failure modes or port operability. The process is even more complex if the 123 probabilistic verification is also performed including climate change projections.

124 <u>To our knowledge</u>, Oonly one study has evaluated the effect of climate change in port operability 125 caused by wave agitation due to SLR (three values) and wave changes from one GCM. A

- metamodel based on the 40 simulations of wave propagation inside the port is/was? applied [8].
- 127 The Linoperability time wasis obtained as the sum of the frequencies of occurrence from the wave
- sets exceeding a fixed threshold. No Any-assessment of port operation downtimes due to wave

agitation has been performed using a Monte-Carlo approach, <u>nor moreover</u>-including climate
 change.

In this work, we propose an integrated methodology for very long-term probabilistic assessments and the second sec 131 of port operability due to wave agitation, including the potential effects of climate change-is 132 proposed. Only port operability due to wave agitation wasis considered in order to simplify the 133 134 methodology's -description, of the methodology but the method it ccan be easily extended/used for-to other applications. The probabilistic verification comprises the use of: 1) a stochastic 135 generator which simulates synthetic multivariate forcing conditions at the entrance of the harbor; 136 and 2) a metamodel to transfer these marine conditions inside the port. Synthetic hourly conditions 137 138 of wave agitation under at present and future climate conditionss wereare evaluated to obtain a probabilistic characterization of port operability and to assess changes due to climate change. 139 140 Probabilistic sea level rise (SLR) scenarios wereare considered to account for SLR uncertainty in the evaluation of future operation downtimes. The application of the methodology wasis 141 particularized to a regional fishing port currently experiencing recurrent downtimes. 142

The paper is organized as follows: section 2 describes the study area used as a pilot case; section 3 presents the databases required for the application of the methodology and section 4 provides extensive details on the overall methodology which combines a weather generator and a metamodel and describes the impact of climate change on port operations. The application <u>of the</u> <u>methodology</u> to the regional port is presented throughout sections 2–4 in order to facilitate\_its <u>understanding the understanding of the methodology</u>. Finally, section 5 summarizes and concludes the work.

150

## 151 2 Study area

The Port of Candás (43° 35, 3' N; 5° 45, 5' W) is located in the region of Asturias (, which is 152 located in northwest Spain), and bordered to the north by the Cantabrian Sea to the north. The 153 154 activities of the port are fishing and recreation (see Figure 1). The water depth in the inner harbor 155 varies between 1 and 3 m (Figure 2). Figure 2 shows the bathymetry of the harbor area. The main 156 breakwater has a trapezoidal cross-section consisting of built of an outer layer of 23 tons concrete 157 cubes, a secondary layer with 2-3 tons of gravel, a 50-1000 kg rubble layer and a core. A concrete 158 159 crown wall lies is located on the top of the rubble mound breakwater with aits crest level of at 11.50 m. The geometry and materials of the different natural and artificial structures of the port's 160 inner boundaries causedetermine thatchanges in wave reflection changes along these boundaries 161 atfor different water levels. In/For this approach/ case study?? these variations wereare included 162 in the agitation modelling by using different reflection coefficients along the berths and docks for 163 the four sea levels considered (see Figure 2). Specifically, the following reflection coefficients 164 werehave been considered, according to the typology they represent: dissipative beach (Kr=0.15), 165 reflecting beach (Kr=0.20), rubble-mound breakwater (Kr=0.40), cliff (Kr=0.60) and vertical 166

167 <u>wharf (Kr=0.9)</u>. For low and mean tide reflection coefficients <u>wereare</u> kept constant.



Figure 1. Location of the Port of Candás <u>in northernat the north of</u> Spain.



171

Figure 2. Upper figure: Bathymetry of the study area area of study (depth in meters). Lower figures: Reflection coefficients adopted/reached? along the port boundaries under different sea levels: low and mean tide, high tide and high tide + SLR. Kr=0.15 for dissipative beach, Kr=0.20 for reflecting beach, Kr=0.40 for rubble-mound breakwater, Kr=0.60 for cliff and Kr=0.9 for

- 176 vertical wharf.
- 177

## 178 **3 Databases**

179 Sea level pressure fields of the Climate Forecast System Reanalysis (CFSR and CFSRv2; [24])

180 were are-used to define the predictor of the statistical models to be explained in section 4. The 181 temporal coverage spanneds from 1979 to 2013, with an hourly temporal resolution and a  $0.5^{\circ}$ 

182 spatial resolution.

The historical wave information used in this work iswas the high resolution coastal wave database Downscaled Ocean Waves (DOW, [25]), with a low resolution mesh resolution of 0.01°x0.008° 185 (low-resolution meshes), and with several nested meshes reaching a maximum resolution of 200

- 186 m. This database <u>wasis</u> generated using a hybrid downscaling methodology which combines
- 187 statistical techniques and dynamical simulations. The Global Ocean Waves database (GOW, [26]) 188 at a regional scale wasis used at the regional scale as wave forcing to generate the coastal wave

reanalysis. The SeaWind database, generated by performing a dynamical downscaling of the

NCEP/NCAR wind reanalysis at a spatial scale of 30 km [27], wasis used as wind forcing. The

- 191 results of these procedures/models/analyses Outputs provided the following hourly sea state
- parameters from 1948 to 2014: significant wave height  $(H_s)$ , mean period  $(T_m)$ , peak period  $(T_p)$
- 193 and wave direction ( $\boldsymbol{\theta}$ ).
- 194 The 62-year (1948–2014) high-resolution hindcast of the meteorological sea level component

195 (storm surge, SS) (GOS 1.1; [28]) <u>was has been</u> used to determine for the historical storm surge

196 data. The GOS 1.1 database <u>washas been</u> developed for Southern Europe using the Regional Ocean

197 Model System (ROMS) with a horizontal resolution of  $1/8^{\circ}(\sim 14 \text{ km})$ .

The astronomical tide (AT) <u>wasis</u> reconstructed <u>on an hourly basis</u> at <u>a 0.25°</u> spatial resolution <u>of</u> <u>0.25°</u>, using harmonic analyse <u>is onfrom</u> the outcomes of the global model of ocean tides (TPXO7.2) that assimilates data from TOPEX/Poseidon missions and tidal gauges for the common period of waves and storm surge.

The regional SLR by 2100 for RCP8.5 scenarios <u>wasis</u> extracted from <u>the</u> global projections of regional mean sea level<u>values</u> obtained by [29] using a dynamical modeling <u>approach</u> that incorporates regional contributions of land ice, groundwater depletion and glacial isostatic adjustment, including gravitational effects due to mass redistribution.

206

# 207 **4 Methodology and results**

- 208 The methodology described in Figure 3 is composed of two main parts:
- A weather generator to derive hourly multivariate marine conditions outside the port.

• A metamodel to transfer hourly marine conditions outside the port<sub>5</sub> as generated in the previous step1, to the inner harbor<sub>a</sub> in order to obtain wave agitation.

The definition of the stochastic generator requires historical information of the forcing conditions 212 outside the port. The climate emulator based on weather patterns for modelling daily multivariate 213 214 events [20] wasis extended to simulate hourly waves and storm surges at the entrance of the port. 215 The model is based on a predictor-to-predictand synoptic regression-guided classification [9]. grouping marine conditions according to similar generating meteorological processes, called 216 weather types (WTs). This method ensures that the predictand within each WT is independent and 217 identically distributed for the applicability of Gaussian copulas to model the dependence between 218 219 variables. Besides, the method captures the climate's non-stationary characteristics based on by means of the variability of WTs over time. A Monte Carlo approximation is applied to 220 stochastically simulate large samples of hourly conditions at the entrance of the harbor. 221

For the second step, a metamodel based on a hybrid downscaling methodology (a combination of

dynamical and statistical downscaling) developed to generate high resolution nearshore wave

reanalysis databases [25] <u>wasis</u> adopted. <u>Specifically, Aa</u> number of representative sea states <u>wasis</u>

propagated using a model solving the elliptic mild slope (MSP, [30]) and the time series reconstructed of nearshore waves were reconstructed by means of an interpolation technique. The

227 way in which the number of simulations wasis selected from the synthetic data ensureds the

228 coverage of the new multivariate space of climate drivers. The probabilistic assessment of current

- port operability due to wave agitation wasis obtained by reconstructing the significant wave height
- 230 inside the port for each simulated hourly condition at the entrance of the harbor for the present
- 231 climate.
- 232 To assess climate change impacts on port agitation, climate change can be introduced in the

233 weather generator by means of future WT probabilities that can be reflected as changes in waves

- and storm surges and SLR added to the <u>sea level</u> time series <u>of sea level</u>. The metamodel has to be
- updated to take into account climate change in those cases selected to be modelled as well as the
- effect of SLR on the reflection coefficients to be used in the wave agitation model.





Figure 3. Probabilistic methodology which combines a weather generator and a metamodel <u>tofor</u> asses<u>sing</u> port operability due to wave agitation <u>underfor</u> present and future conditions.

240

Figure 4 shows the time series for years 2013 and 2014 and the distribution of  $H_s$ - $\theta$  and  $T_p$ - $\theta$  of

the forcing conditions <u>occurring</u> outside the port, obtained from the databases described in section

243 2. ForcingThese conditions are defined outside the port were defined at about 6.0 m depth. Wave

climate at this location has suffered an intense refraction due to the protection effect of Cape Peñas

245 (Figure 1) resulting in wave energy concentration in the N-E sector. The maximum significant

wave height is limited to 4.5 m while peak periods reach values of 20 s which can be combined with storm surges of almost 0.5 m and with high spring tides over 2.0 m.



248

Figure 4. <u> $H_s$ ,  $T_p$ , SS and AT values for two years within the time series Example of two years of time series ( $H_s$ ,  $T_p$ , SS and AT) at the entrance of the harbor (left panels).  $H_s$  and  $T_p$  roses (right panels).</u>

252

#### 4.1 Weather generator

A weather-type framework wasis used to model the nonstationary behavior of the local 254 multivariate predictand ( $H_s$ ,  $T_m$ ,  $T_p$ ,  $\theta$  and SS) related with large-scale predictors (sea level 255 pressure, SLP). The Ddaily predictor wasis classified into a discrete number of weather patterns 256 257 (WTs) according to their? synoptic similarity. HThe hourly multivariate events wereare modelled using a marginal distribution for each predictand variable and a Gaussian copula within each WT. 258 The stochastic generator follows is composed of similar steps as to the one developed by [20] for 259 multivariate extremes, except in this case the extremal index is not required. The five steps 260 involved in this model are: 1) To Collect and pre-process historical data of the predictor (sea level 261 pressure, SLP) and predictands ( $H_s$ ,  $T_m$ ,  $T_p$ ,  $\theta$  and SS). 2) Define <u>WTs</u>weather types using a semi-262 guided classification [31]. 3) Fit a stationary model (e.g. Lognormal, Generalized Extreme Value) 263 264 to each variable of the multivariate predictand ( $H_s$ ,  $T_m$ , SS outside the port) associated with each WT weather type. 4) Model the dependence between predictand variables within each weather type 265

using a Gaussian copula. 5) Generate synthetic multivariate hourly conditions taking into account
 the monthly WT probability and dependence structure associated with each WT.

The spatial domain of the predictor should cover the oceanic region responsible for generatingion 268 of-waves arriving at eacha location of interest. The temporal coverage (recent history) should 269 account for the wave travel time from generation to the target location. Based on previous works, 270  $\mp$ the semi-supervised WTs of the grid node from the global collection of WTs at a  $1.0^{\circ} \times 1.0^{\circ}$ 271 resolution, generated to obtain global wave projections [9], at a location closest to the study port 272 273 of study, wasis used to develop the weather generator (steps 1 and 2 in this section). The predictor definition (spatial domain and temporal coverage) correspondeds to the subdomain covering which 274 covers- the North Atlantic Ocean (from an ocean the division of the ocean based on a global wave 275 genesis characterization). The predictor wasis defined as the 3-daily mean SLP and 3-daily mean 276 SLPG (squared SLP gradients), calculated daily throughout the historical time period. More details 277 regardingabout this characterization and the WT collection can be found in [9]. A regression 278 guided classification was applied to a combination of the weighted predictor and predictand 279 estimations from a regression model linking the SLP fields with local marine conditions. The level 280 of influence of the wave and storm surge data wasis controlled by a simple weighting factor which 281 balances the loss/gain of predictor/predictand representativeness. A factor equal to 0.6 wasis 282 implemented based on previous sensitivity analyseis. A better grouping of the predictand wasis 283 obtained due to a stronger relation of the WTs with the local marine climate conditions. 284

The long-term marginal distributions (step 3) of hourly  $H_s$ ,  $T_m$ , and SS outside the port within each WT wereare fitted to a generalized extreme value (GEV) distribution, -a Lognormal distribution or Unified Distribution Model [32], obtaining the best fit of the central regime with a GEV. The empirical distribution wasis used for the wave direction variable. An heteroscedastic model between  $T_p$  and  $T_m$  wasis fitted within each WT. <u>Tp</u> wasis considered to be normally distributed with parameters mean and variance being a function of  $T_m$  (polynomials with unknown degree). A Gaussian Copula iwass used to model the dependence between  $H_s$ ,  $T_m$ , SS and  $\theta$  (step 4).

The Monte Carlo sampling procedure used to generate synthetic marine conditions (step 5) 292 requires the following phases: i) Sample a daily WT from a Generalized Bernoulli distribution due 293 to the categorical choice of one of the N=100 WTs. ii) Randomly Generate randomly 24 hourly 294 synthetic  $H_s$ ,  $T_m$ ,  $\theta$  and SS using the Gaussian copula and the marginal fits associated with the daily 295 simulated WT; iii) Sample 24 hourly  $T_p$  from the heteroscedastic model between  $T_p$  and  $T_m$ 296 297 associated with the daily WT; iv) Independently sample 24 hourly values of astronomical tide from its monthly empirical distribution. The process is repeated until a synthetic 90-year time series of 298 90-years hourly time series of multivariate forcing marine conditions is obtained. 299

One thousand, 90-year long, new time series of  $H_s$ ,  $T_m$ ,  $T_p$ ,  $\theta$ , SS and AT wereare simulated with the <u>emulator</u> previously fitted <u>emulator</u>. Each series <u>wasis</u> generated with a different set of parameters, randomly taken from the parameters sample obtained considering a Gaussian distribution. Scatter plots of the five sea-storm variables are shown in Figure 5. Monte Carlo

304 simulations (1000 samples of 50 years of hourly data, <u>comparable to thein order to compare with</u>

305 50 years of historical dataperiod) are shown as grey dots and historical data as black dots. The

306 large multivariate sample of hourly forcing conditions captures the characteristics of dependencies

307 <u>among between the variables</u>. <u>Wave breaking and wave steepness limite the Mmaximum simulated</u>

308 wave height-is limited to 5.0 m and mMaximum simulated wave period was is-limited to 25-s.

309 The effect of the imposed physical limitations of wave slope-imposed can be observed in the 310 correct reproduction of the relation between wave heights and small wave periods. Figure 6 shows the joint probability density functions of  $(H_s, T_p)$ ,  $(H_s, \theta)$ ,  $(H_s, SS)$  and  $(T_m, T_p)$  obtained from the 311 historical series (blue lines) and from the simulated series (dashed lines). The simulated series are 312 able to reproduce the main features of the original bivariate distributions. They fail in representing 313 some details of the distributions, as the clearcertain dependence between wave heights around 1.0 314 m and low peak periods. 315 316 317 318 SS (m) θ(°) 0 200 250 300 150 350 0.5 0.75 Hs (m) 350 300 250 () θ 150 100 50 0.5 SS (m) 319 -0.5

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**Figure 5.** Scatter plots of marine climate ( $H_s$ ,  $T_p$ ,  $\theta$ , SS, AT) at the entrance of the port. Historical data: black-grey dots; Monte Carlo simulations (1000 samples of 50 years of hourly data)simulated data: grey black dots. 





Figure 6. Joint probability density function of the hourly forcing conditions. Blue solid lines represent the results obtained from the historical data and black dashed lines represent the simulated data generated using the weather generator.

Regarding future synthetic time series in a future period (e.g. in the period, 2010-2100), climate 332 333 change can be introduced taking into account changes in the storminess by means of future WT probabilities from GCMs and the increase inof the mean sea level. Robust multi-model ensemble 334 projections at high spatial resolutions (0.01°x0.008° using DOW at the reference database) 335 measured over the whole century (2010–2099) were estimated along the northern coast of Spain 336 [33]. Future wave and storm surge projections were statistically downscaled using a weather-type 337 approach [34] for the same 40 GCMs as in the regional wave projections made in Europe [35]. 338 339 The statistical relationship was is established as similarly into the first steps of the weather generator. In this case, however, the empirical probability distribution of each sea state parameter 340 (e.g., significant wave height) associated with each WT wasis calculated. The distribution of this 341 variable for a certain time period can be estimated as the sum of the probability of each WT during 342 that period multiplied by the corresponding empirical distribution. Different statistics (e.g., mean, 343 95th percentile) can be derived from the estimated distribution. One of the advantages of this 344

345 statistical downscaling methodology is that the scale representativeness of the projections depends

on the underlying historical wave databases used as a reference [9]. Figure 7 shows the multimodel ensemble projections of the annual mean and the 95<sup>th</sup> percentile of the significant wave height, the

347 ensemble projections of the annual mean and the 95<sup>th</sup> percentile of the significant wave height, the 348 mean period and the 95<sup>th</sup> percentile of the storm surge in the area surrounding around the study

port for the time-period 2070–2099 compared with respect to the 1979–2010 period under the

350 RCP8.5 scenario. Box plots illustrated inform about the uncertainty inherent in of the future

changes obtained from the 40 GCMs. The outcomes reveal slight decreases in surges and wave

heights and periods. These changes are assumed to be negligible compared to the

353 SLR in the wave propagation inside the port. Indeed Moreover, the decreasing waves and storm

354 surge <u>resulting from according to</u> these expected changes would <u>underestimate the need for the</u>

355 <u>assessment of port operation downtimes.</u> reduce the safety level of the port exploitation.



Figure 7. Regional multimodel projections (RCP8.5, 2070–2099 with respect to 1979–2005) for the mean and the 95<sup>th</sup> percentile of <del>the</del>-wave significant wave height, <u>the-mean wave period</u> and the 95<sup>th</sup> percentile of <del>the</del>-storm surge along the coastline <u>surrounding around</u> the study port.

Following the approach proposed by [36] to account for the SLR uuncertainty around SLR in the 362 assessment of flooding risk, a lognormal distribution wasis fitted with thea mean and standard 363 deviation of the from regional projections produced by [29] for the RCP8.5 scenarios in 2100 (i.e., 364  $0.63 \pm 0.20$  m at the study area) for the RCP8.5 scenarios. The Llognormal distribution is 365 considered as the most likely distribution representing future sea level rise/-SLR [37], although 366 increased rates of ice sheet loss are/were not included in this study. The deciles from fitted 367 lognormal distributions split the SLR data set in from each horizon year into ten equally probable 368 parts-with equal probability, being the 2100 deciles being: 0.377 m; 0.454 m; 0.507 m; 0.553 m; 369 0.599 m; 0.646 m; 0.699 m; 0.763 m; 0.852 m; and 1.025 m, respectively. Ten curves wereare 370 derived from the local RCP8.5 SLR values in 2025, 2050 and 2100 using a second order 371 polynomial function in order to adopt the shape of those provided by the from IPCC [38]. Hourly 372 SLR time series (2010-2100) derived from these curves wereare added to the synthetic sea level 373

time series (defined as the sum of storm surge and astronomical tide) to define the future forcing

- 375 conditions of port agitation.
- 376

# 377 4.2 Metamodel

The steps <u>followed</u> to define the metamodel <u>used</u> to transform all the synthetic forcing conditions outside the port <u>wereare</u>: 1) selection of a limited number of cases <u>comprising which are</u> the most representative <u>scenarios</u> of waves and sea level fluctuations (storm-surge, astronomical tide and sea level rise) outside the port; 2) a wave agitation strategy to propagate the selected sea states from the entrance of the bay towards the inner harbo<del>u</del>r zone; 3) reconstruction of the time series of significant wave heights inside the port.

A subset of sea states (M=500) representative of the-marine conditions outside the port wasis 384 selected using the maximum dissimilarity algorithm (MDA, [12]). The MDA identifies a subset 385 comprising the most dissimilar data in a database. The selection starts by initializing the subset 386 through the transference of one vector from the data sample. The remaining st of the elements are 387 selected iteratively, transferring the most dissimilar one from the remaining data in the database to 388 the subset.- Figure 8 shows the distribution of the selected subset from the MDA (larger red dots) 389 over the full multivariate parameter space ( $H_s$ ,  $T_p$ ,  $\theta$  and sea level) covered by the Monte Carlo 390 realizations (grey dots). The multivariate subset is distributed evenly across the space covering the 391 potential combinations between the four variables with some points selected in the outline of the 392 393 data space, which contributinges to an accurate reconstructions of wave agitation conditions inside

the port using the proposed metamodel.

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Figure 8. Scatter plots of simulated data (grey dots) and the selected cases using the MDA (red dots in red).

396

The MSP numerical model [30] wasis used for wave agitation simulations. This model is able to 400 solve the wave propagation towards and into the harbour, taking into account the refraction, 401 diffraction, wave breaking and partial reflection imposed by the natural and artificial structures 402 (quays, basins, breakwaters, etc.) and real bathymetry contours. The model provides (2DH) 403 significant wave maps along the whole numerical domain. A complete spectral sea-state 404 propagation strategy [389] based on the invocation of a pre-calculated monochromatic wave 405 catalogue wasis applied to noticeably reduce the CPU-effort to propagate real wave spectra 406 towards any inner control point. This technique is based on a three-step method: 407

The selection of N monochromatic wave conditions (the combination of periods T and directions ??) by collapsing the 4D-hypermatrix [frequency, direction, energy, time] for the whole
 wave hindcast used, into a single resulting matrix representings the historical energy packs
 available in the study zone (for a typical 35 frequency x 72 direction spectrum matrix, and taking into account real/theoretical frequency and direction spreading factors for each hour). N can adopt

- 413 values from \_30% to 60% of the total matrix size used, depending on the geographical location of
   414 the harbor/ outer wave climate.
- 415 2. The numerical propagation of each N monochromatic waves (using a constant wave height
- 416 <u>H, because of the linear nature of the model used) and for the different sea levels considered.</u>
- 417 <u>3.</u> The aggregation of any spectrum by adding all the individual energy packs that defines it.
- 418 For this study additional considerations were established:
- 419 <u>4. Four water levels were used (total Nx4 monochromatic cases) (three to cover the astronomical tide range and one as expected upper SLR).</u>
- 421 <u>5.</u> Changes in reflection coefficients in the model's setup (as described in the study area
   422 <u>section</u>) due to SLR.
- 423 Monochromatic wave conditions, covering all wave periods and directions physically available at 424 the entrance of the bay, are propagated inside the port using the MSP model at four water levels 425 (to cover the astronomical tide range and an expected upper sea level rise). Changes in reflection 426 coefficients due to SLR are implemented in the wave model setup (as described in the study area 427 section). For each of the real sea state conditions selected, wave agitation inside the port is 428 reconstructed by linear superposition of the results corresponding to monochromatic waves and
- 429 linear interpolation at the corresponding sea level.
- 430 This technique, besides achieving a radical CPU-time reduction, enables to rapidly -include any future scenario needed or sensitivity analysis required, as well as changes in one or many spectrum 431 variables due to climate change (energy, frequency, direction and its frequency-directional 432 433 spreading). OIn the other hand, this technique could over-predict wave-shoaling effects, especially for shallow bathymetry zones. Thus, it should be used with caution if non-linear wave-wave 434 interactions are expected in the study zone, especially for wave breaking related processes and 435 shoaling. This drawback is minimized for open harbors, with (in general) quasi-constant/ mild 436 437 bathymetry configurations within the basins and outer zones, as showned in [39]. Figure 9 shows an example of a wave agitation map inside the port for conditions outside the port defined by 438  $H_s=7.2$  m;  $T_p=15.8$  s;  $\theta = 54.5^{\circ}$  and sea level=3.63 m. 439



441 **Figure 9**. Wave agitation map for the following marine conditions outside the port:  $H_s$ =7.2 m; 442  $T_p$ =15.8 s;  $\theta$  =54.5° and sea level=3.63 m.

The significant wave height time series inside the port are reconstructed using the 443 multidimensional interpolation technique of radial basis functions (RBF, [3940]). The RBFs 444 enable a statistical relationship to be defined between the marine parameters characterizing the 445 forcing conditions and the wave height inside the port from the results of the selected cases. The 446 RBF interpolation method defines the function to be approximated by means of a weighted sum 447 of radially symmetric basic functions located at the data points where the results are available. A 448 more detailed description of these statistical tools implemented in the proposed hybrid 449 methodology can be found in [12]. 450

451

## 452 4.3 Results

The synthetic historical time series of marine conditions (waves, storm surge, and astronomical tide) outside the port <u>wereare</u> transferred inside the port using the corresponding RBF. The synthetic time series outside the port <u>wasare</u> transformed to the future period 2010-2099 adding the corresponding?? SLR to each hourly sea level. Their corresponding wave height inside the port <u>was is</u>-reconstructed applying the RBF.

The annual operability or the hours of non-operability <u>areis some one</u> of the basic <u>port</u> design criteria for <u>ports</u> stipulated <u>given</u> by national and/or international standards (such as ROM or

460 PIANC). In this example, hours of non-operability wereare calculated from each time series as the

hours exceeding a certain threshold of  $H_s$  inside the port. Here, Aa threshold equal toof 0.4 m wasis

462 applied, as suggested in the Spanish Recommendations for Maritime Structures for Efishing Pports

463 (ROM 3.1-99, [40<u>41</u>]).

Figure 10 shows the historical and future empirical cumulative distributions of significant wave 464 height, Hs, inside the port in Areas 1 and 2 from the one thousand synthetic time series. Fifty50-465 year long time series of forcing conditions wereare considered in the assessment of the port's 466 downtimes since because the useful life of the Port of Candás is established in 50 years. The future 467 distribution wasis based on the thousand synthetic future hourly time series from 2050-2099 468 obtained for the ten SLR scenarios sampled from a lognormal distribution of the RCP8.5 SLR 469 projections. The future empirical distributions for the ten SLR scenarios wereare represented in a 470 471 yellow-red scale corresponding to the lowest-to the highest decile, respectively. The probability of a significant wave height lower than 0.4 m (non-operability threshold for fishing ports) is lower 472 473 the higher the as the SLR is higher (see the zoomed image of the empirical cumulative distribution between 0.35 to 0.45 m in Figure 10). 474



475

Figure 10. Historical empirical cumulative distribution (in blue) and future empirical cumulative distributions for the ten SLR scenarios (in yellow-red scale) of significant wave height inside the port in Area 1 and Area 2.

479

480 Hours of non-operability wereare calculated from the probability (p) obtained for a threshold of 0.4 m as  $(1-p) \times 365 \times 24$ . The probabilistic distributions of non-operability hours at present (blue) 481 climate (in blue) and in the future period (2050-2099, (iin red) climate conditions are shown in 482 Figure 11, Future The distributions of non-operability for each RCP8.5 SLR scenario in the future 483 are displayed (dashed lines in the yellow-red color scale) with the ensemble mean future 484 probabilistic distribution of non-operability (in red). The ensemble mean distribution wasis 485 486 obtained by adding up the distribution for each of the ten SLR scenarios multipliede by 0.1 (the ten SLR scenarios are sampled with an equal probability). It can be noted that hours of non-487 operability do increase from present to future conditions for both areas. 488

489 The probabilistic distribution of non-operability hours <u>under current at present climate conditions</u>

490 represents the uncertainty associated with the historical forcing conditions outside the port. The

491 ensemble mean future distribution integrates the uncertainty associated with the RCP8.5 SLR

scenarios and the uncertainty due to the forcing conditions outside the port (distributions of non-

493 operability hours of ten future RCP8.5 SLR scenarios). Besides,-higher non-operability hours, as





Figure 11. Probabilistic distributions of non-operability hours due to wave agitation in Areas 1 and 2 inside the port in <u>current the present climate</u> (in blue) and <u>in the future (2050-2099) climate</u> <u>conditions</u> in each SLR scenario (in yellow-red scale) and the future ensemble mean distribution of non-operability hours (<u>thick red line??in red</u>).

504 505

499

506 One additional way to summarize and compare the results obtained is displayed in Table 1. Each 507 of the present and future distributions of non-operability hours is fitted to a lognormal distribution. 508 The mean, standard deviation and coefficient of variation are calculated and shown in the table. In both Areas 1 and 2, the future mean values for non-operability hours are drastically increased (from 509 510 198.90 to 332.13 in Area 1 and from 313.62 to 475.14 in Area 2) due to a mean SLR of 0.257 m by 2050 and 0.634 m by 2100. The latter increase is due to non-linear interactions between waves 511 512 and sea level, and changes in the reflection coefficients associated to SLR. Regarding the nondimensional coefficient of variation, the uncertainty associated with in non-operability hours 513 514 increases from about 1.3 % (0.0132 in Area 1 and 0.0129 in Area 2) under currentin the present conditions, to around 10 % (0.1163 in Area 1 and 0.1044 in Area 2) in the-future ones. These 515 516 results differ from the future SLR coefficient of variations (26.7% by 2050 and 31.1% by 2100), indicating that the magnitude of the SLR uncertainties are reflected toin a lower degree in the 517 magnitude of the uncertainty of non-operability hours. 518

	RCP8.5 SLR (m)		Non-operability (hours)				
			Area 1		Area 2		
	2050	2100	Present	Future (2050-2100)	Present	Future (2050-2100)	
Mean	0.257	0.634	198.90	332.13	313.62	475.14	
Std	0.069	0.197	2.625	38.61	4.057	49.604	
CV (std/mean)	0.267	0.311	0.0132	0.1163	0.0129	0.1044	

519 Table 1: Mean, Standard Deviation and Coefficient of Variation of the lognormal distribution of

2050 SLR and 2100 SLR predictions? and the lognormal distribution of the non-operability hours

521 for the present and future period in Areas 1 and Area 2.

522

523 NThe non-operability hours during the useful life of the infrastructure/port taking into account climate change washas been calculated along the 21st century adding the evolution of the SLR to 524 the hourly time series of sea level. In the previous analysis of the impact of climate change in the 525 526 port's operability, future of non-operability hours wereare calculated for a useful life of 50 years. from 2050 to 2100, to obtain more significant changes. However, the assessment of port operability 527 should be adjusted fit to the projected useful life of the infrastructure, as of starting at the moment 528 529 of its construction. Figure 12 shows the interannual variability of the ensemble's mean probability of non-operability hours from 2010 to 2099 in Areas 1 and Area-2 based on from the ten SLR 530 scenarios. First, the empirical distribution of non-operability hours of non-operability was has been 531 532 calculated on a yearly basis every year for each of the 10 RCP8.5 SLR scenarios considered. The mMean sea level rise rates werehave been determined fitting a second order polynomial to the 533 deciles from the local SLR lognormal distribution in 2025, 2050 and 2100. Afterwards, the 534 535 ensemble mean distribution of non-operability hours was has been calculated every year. An average moving mean of ten years washas been applied. A linear trend of the mean hours of non-536 operability along the 21st century can be observed in Figure 12 (e.g., downtime increases from 537 320 hours in 2010 to 510 hours in 2100). The dispersion of the empirical density distribution rises 538 along the 21st century due to a broader uncertainty of the SLR scenarios as the horizon increases. 539 At the beginning of the 21<sup>st</sup> XXI century, the SLR distribution spread wasis limited which-it is 540 reflected in a narrow ensemble mean distribution of non-operability hours (i.e., high probability 541 centered in the mean value). However, the SLR distribution-is broadened along the 21st century, 542 543 increasing which makes the ensemble mean distribution of hours of non-operability wider (e.g., downtime hours vary from 290 to 400 hours in 2010 and from around 400 to 800 in 2100). An 544 average moving mean of ten years has been applied. It can be observed not only how the mean 545 hours of non-operability increases during the 21st century, also the dispersion of the empirical 546 density distribution rises along the 21st century due to a broader uncertainty of the SLR scenarios 547 548 as the horizon increases. Changes in The changes of hours of non-operability hour values are more

- significant after 2050 due to a more pronounce<u>d</u> acceleration of SLR <u>as of from t</u>the second half
- 550 of the 21st century.



**Figure 12.** Interannual ensemble mean probability of non-operability hours from 2010 to 2099 in Area 1 and Area2 taking into account the increase inof SLR uncertainty along the 21<sup>st</sup> century.

554

#### 555 **5 Summary and conclusions**

A hybrid statistical-dynamical framework <u>wasis</u> developed with two main purposes: 1) to provide a probabilistic evaluation of port operability to <u>assess</u> <u>ensure thea</u> <u>a</u> minimum level of downtime of the port; 2) -to introduce climate change in the assessment of port operability during its useful life.

The methodology is strongly dependent on the multivariate nature of climate drivers of wave agitation <u>such</u> as <u>the</u> combination of waves and sea levels and the availability of these forcings outside the port. Therefore, the following <u>requirements should be met-is required</u>: 1) the use of a stochastic generator to model the dependence between multivariate conditions; 2) the application of <u>a</u> numerical modelling approach to propagate wave offshore conditions inside the port.

565 <u>HenceFor said reasons</u>, the methodology includes: 1) A weather generator based on WTs to take 566 into account future climate variability through WT probability changes linked to changes in 567 climate drivers (waves and storm surges); 2) A metamodel based on a catalog of wave propagations and a multidimensional non-linear interpolation to reconstruct hourly significant wave height time series inside the port with a<u>n-similar</u> accuracy <u>similar to that of the as</u> numerical simulations.

The study case study was is focused on port operability due to wave agitation. The methodology 570 allows to transfer inside the port thousands of synthetic time series of at present elimate and in the 571 future climate conditions inside the port in order to carry out a probabilistic analysis of port 572 operability. Future changes inof non-operability are expressed including both uncertainties 573 associated with marine conditions outside the port and SLR. Climate induced changes in waves 574 and storm surge are considered to be negligible due to the projections obtained in the study area. 575 Uncertainty of forcing conditions outside the port wasis quantified through the use of a weather 576 generator that allows to generate synthetic time series. SLR uncertainty wasis introduced equally 577 by sampling its probability distribution in several horizons, while-and hourly SLR time series 578 arewere added to the synthetic sea level fluctuations to define the future forcing conditions outside 579 the port. SLR uncertainty wasis integrated in the future non-operability evaluation joining the 580 contribution of each sampled SLR scenario with its corresponding probability. 581

Obtaining the future distribution of non-operability hours allows calculating the future probability associated with the non-operability exceedance hours threshold established in ports design recommendations (i.e. ROM 3.1-99, [401]) <u>during their for its</u> useful life. The proposed hybrid methodology produces this very useful and relevant outcome to define a specific acceptable operability risk and can be used as a design criteri<u>ona in of a</u> new coastal infrastructures or for climate change adaptation plans.

27

Although, for this specific pilot case, climate induced changes on waves and storm surges, have been neglected due to <u>theirits</u> small values, non-linear feedbacks induced by SLR that may produce an amplification of wave conditions in shallow waters [42+] have been introduced in the wave

agitation modeling. Future hourly sea conditions are transformed from the harbor's entrance to

592 inside the port considering the non-linearities between tides, surges, waves and SLR. Changes in

the reflection coefficient inside the port due to changes in sea level have also been implemented

in the <u>wave agitation</u> simulation of wave agitation.

The proposed methodology presents several limitations. The synthetic marine conditions are generated without modelling the dependence time structure dependence, which would allow <u>performingto perform</u> an analysis of the non-operability's persistence. Besides, this version of the climate emulator is not useful for the analysis of extreme conditions. <u>SThe synthetic extreme</u> events are not time independent and their frequency could be overestimated. Nevertheless, our objective <u>wasis</u> focused on port operability which sh<u>ouldall</u> not be conditioned by extreme events.

601 The methodology presented can be extended <u>tofor</u> further applications such as coastal 602 infrastructure reliability or operability for other functional parameters or marine operations by 603 tailoring the weather generator and <u>a</u> selecting the most appropriate numerical model.

604

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