

# Measuring energy in dynamic probing

Ibáñez, S.J.

INGITER S.L.-University of Burgos, Burgos, Spain

Sagaseta, C.

University of Cantabria, Santander, Spain

López, V.

INGITER S.L.-University of Burgos, Burgos, Spain

**ABSTRACT:** The penetration in these tests is intimately related to the potential energy of the hammer (nominal energy). So, it is necessary to understand correctly and deeply how this energy is transferred to the drive rods and how it is transmitted through the rods to the cone, to realize how the real behavior in these tests is. The penetration length really depends on the energy, not on that nominal energy, but on a portion of that energy that is effectively transferred to the rods (*ENTHRU*) and, in order to be more precise, the energy that reaches the cone (*ENTHRU<sub>cone</sub>*). The *ENTHRU* is measured by means of monitoring the upper part of the drive rods (close to the anvil). To calculate the *ENTHRU<sub>cone</sub>* it is necessary to correct the *ENTHRU* in three ways. First it is needed to take into account the energy loss in the energy transmission through the rods, and the energy loss due to the skin friction of the drive rods with the soil around them. It is also necessary to add the energy due to the rod weight that is penetrating into the soil.

**KEY WORDS:** dynamic, penetration, energy, DPSH

## 1 INTRODUCTION

There are different kinds of penetration tests. First of all, we have the Standard Penetration Test (SPT), maybe the most widespread penetration test. There is lot of literature about measuring energy in these kind of tests. However, there are many other penetration tests that differ a lot from SPT. They are called dynamic probing (DPH, DPSH...).

The SPT tests are performed into a borehole with a sampler, meanwhile dynamic probing tests are performed from the surface with a cone. So the main difference is quite clear: the friction between the drive rods and the soil around them.

## 2 ENERGY EFFICIENCY FOR SPT

Several authors have studied and measured the SPT energy. In this paper we will use one of the latest investigations by Odebrecht & Schnaid, et al (2005).

Housel (1965) was the first who used the word *ENTHRU*, that is the maximum energy transmitted to the rod stem. The *ENTHRU* was measured by means of integrating signals from some accelerometer and some strain gauges located below the anvil, as usual for these kinds of tests.

Odebrecht & Schnaid, et al (2005) found that this *ENTHRU* could be fitted to this equation:

$$ENTHRU = \eta_1 \cdot m_h \cdot g \cdot (h + p) \quad (1)$$

where  $\eta_1$  = hammer efficiency factor;  $m_h$  = hammer mass;  $g$  = gravity acceleration;  $h$  = height of fall (free fall of the hammer after being released); and  $p$  = penetration for one blow.

After analyzing the data from the tests, the value of  $\eta_1$  was calculated as 0.765.

Odebrecht & Schnaid, et al (2005) also inferred the value of the computed sampler energy (*E<sub>sampler</sub>*). The sampler energy is the value of the actual energy that reaches the sampler.

$$E_{sampler} = \eta_3 \cdot [\eta_1 \cdot m_h \cdot g \cdot (h + p) + \eta_2 \cdot m_r \cdot g \cdot p] \quad (2)$$

where  $\eta_3$  = energy efficiency factor;  $\eta_2$  = rod efficiency factor; and  $m_r$  = rod mass.

The experimental data were adjusted and the proposed values for the efficiency factors were:

$$\eta_3 = 1 - 0.0042 \cdot l \quad (3)$$

where  $l$  = rod length.

The value of  $\eta_2$  equals 1.

### 3 DIFFERENCES BETWEEN SPT AND DYNAMIC PROBING

Both kinds of tests are quite similar but there are some differences that will have to yield to different ways of measuring the energy.

The SPT tests are carried out with a sampler that is driven into the soil so, after the tests there is a recovered disturbed sample. The dynamic probing tests use a cone, instead of a sampler. This cone may be either retained (fixed) for recovery or disposable (lost).

Even though both tests are quite similar, in terms of analyzing the energy, there is a mayor difference. As the SPT tests are undertaken inside a borehole there is no friction between the drive rods and the soil around them, whereas in dynamic probing, as the cone is not much wider than the rods, such friction exists.

### 4 MEASURING THE LATERAL FRICTION

The drive rods and the cone will be driven vertically in order to avoid such lateral friction.

The rods will be rotated 1.5 turns or until maximum torque is reached at least every 1 m penetration. The aim of this rotation is to tighten the rod conections and to reduce the skin friction.

Every dynamic probing equipment has a torque measuring device. It is usually a torque wrench or similar measuring device. The rods will be rotated by means of this device so, at the same time, we get the value of the maximum torque.

The frictional force  $F_f$ , necessary to rotate the drive rods, is calculated with the skin resistance, and the lateral area of the drive rods.

$$F_f = \tau \cdot 2 \cdot \pi \cdot r \cdot l \quad (4)$$

where  $\tau$  = skin friction resistance;  $r$  = rod radius; and  $l$  = rod length.

The maximum torque necessary to rotate the drive rods will be the frictional force multiplied by the force arm.

$$T = F_f \cdot r = \tau \cdot 2 \cdot \pi \cdot r^2 \cdot l \quad (5)$$

where  $T$  = maximum torque.

This way, the value of the skin friction is calculated in Equation 6 below:

$$\tau = \frac{T}{2 \cdot \pi \cdot r^2 \cdot l} \quad (6)$$

The energy used to overcome the friction between the drive rods and the soil around them, during the penetration, (frictional energy  $E_f$ ) could be calculated with the Equation 7 below:

$$E_f = F_f \cdot p \quad (7)$$

Substituting the value of  $F_f$  from Equation 4 in Equation 7:

$$E_f = \tau \cdot 2 \cdot \pi \cdot r \cdot l \cdot p \quad (8)$$

And using the value of the skin friction from Equation 6, we get the final Equation 9 below:

$$E_f = \frac{T \cdot p}{r} \quad (9)$$

This Equation 9 was obtained by Dahlberg & Bergdahl (1975).

The aforementioned equation only works assuming that the semi static skin friction during rotation of the drive rods is the same as the dynamic skin friction during penetration.

Due to this difference between skin friction in static and dynamic process, Bergdahl (1979) calculated the part of the total number of blows in a SPT test, that was “used” to overcome that lateral friction. When he calculated that  $N$ , he did not divide it by the energy efficiency, so he was really multiplying that  $N$  by the energy efficiency. This way, he realized that the energy due to lateral friction was the same energy as the calculated in Equation 9, but with a factor of 3.4 and multiplying by the energy efficiency. This means that the dynamic lateral friction resistance in the vertical direction during driving is much greater than the semi static resistance in the horizontal direction during turning of the rods.

This way, we can consider the next equation to get that frictional energy:

$$E_f = \frac{T \cdot p}{r} \cdot 3.4 \cdot \eta \quad (10)$$

where  $\eta$  = energy efficiency.

This efficiency means the percentage out of the nominal energy that really reaches down to the cone.

The nominal energy is defined as the hammer weight multiplied by the height of fall.

From now on, it is defined  $ENTHRU_{cone}$  as the real amount of energy that effectively reaches the cone. It is the equivalent to what Odebrecht & Schnaid, et al (2005) called  $E_{sampler}$  in a SPT test.

$$E_f = \frac{T \cdot p}{r} \cdot 3.4 \cdot \frac{E_{sampler}}{m_h \cdot g \cdot h} \quad (11)$$

### 5 $ENTHRU_{cone}$ IN DYNAMIC PROBING

Using the  $E_{sampler}$  in a SPT (Equation 2) as part of the new  $ENTHRU_{cone}$ , we only need to substract that part of the  $ENTHRU$  that is used to overcome that skin friction ( $E_f$ ). Thus, we get the Equation 12:

$$ENTHRU_{cone} = E_{sampler} - E_f \quad (12)$$

By substituting the value of  $E_{sampler}$  (Equation 2) and the value of  $E_f$  (Equation 11) in Equation 12:

$$ENTHRU_{cone} = \eta_3 \cdot [\eta_1 \cdot m_h \cdot g \cdot (h + p) + m_r \cdot g \cdot p] \cdot \left[ 1 - \frac{T \cdot p \cdot 3.4}{r \cdot m_h \cdot g \cdot h} \right] \quad (13)$$

The value of  $m_r$  is the mass of the rods that includes the anvil mass. We can write the Equation 13:

$$ENTHRU_{cone} = \eta_3 \cdot [\eta_4 \cdot m_h \cdot g \cdot (h + p) + m_a \cdot g \cdot p + m_r \cdot g \cdot p] \cdot \left[ 1 - \frac{T \cdot p \cdot 3.4}{r \cdot m_h \cdot g \cdot h} \right] \quad (14)$$

where  $m_a$  = mass of the anvil where the hammer strikes when falling down; and  $\eta_4$  = energy efficiency factor.

The  $ENTHRU$  measured just below the anvil is:

$$ENTHRU = \eta_4 \cdot m_h \cdot g \cdot (h + p) + m_a \cdot g \cdot p \quad (15)$$

Equation 14 could be rewritten as:

$$ENTHRU_{cone} = \eta_3 \cdot [ENTHRU + m_r \cdot g \cdot p] \cdot \left[ 1 - \frac{T \cdot p \cdot 3.4}{r \cdot m_h \cdot g \cdot h} \right] \quad (16)$$

This different way of measuring the  $ENTHRU$  makes different  $\eta_1$  from  $\eta_4$ . Therefore, in dynamic probing we can not assume the value of  $\eta_4$  as 0.765 as in Odebrecht & Schnaid, et al (2005). This fact will be treated further.

The value of  $\eta_3$ , obtained from Equation 3 is going to be changed. It would be better if this factor had no units, so this factor is changed to a non dimensional factor.

If the last part of this factor is multiplied and divided by the rod diameter:

$$\eta_3 = 1 - 0.0042 \cdot l \cdot \frac{d}{d} \quad (17)$$

where  $d$  = rod diameter.

The rod diameter used by Odebrecht & Schnaid, et al (2005) was the normal diameter for SPT drive rods whose value is  $2.28 \times 10^{-2}$  m.

$$\eta_3 = 1 - 0.0042 \cdot l \cdot \frac{2.28 \cdot 10^{-2}}{2 \cdot r} = 1 - 4.8 \cdot 10^{-5} \cdot \frac{l}{r} \quad (18)$$

0.765, as Odebrecht & Schnaid, et al (2005) calculated for SPT.

## 6.1 Instrumentation

The top part of the rods was instrumented by means of four strain gauges and two accelerometers as it can be observed in Figure 1.

The instruments were located  $32.6 \times 10^{-2}$  m below the point of contact between the hammer and the anvil. This length was chosen in compliance with EN ISO 22476-2:2005. It is necessary that the instrumented section of rod is positioned at a distance greater than 10 times the rod diameter below the point of hammer impact on the anvil.

Each of the four strain gauges was fixed and attached to the rod, and was independent from the rest of the strain gauges. They were assembled as 4 different quarter Wheatstone bridges.

The two ICP piezoelectric accelerometers were mounted diametrically opposite on little steel pieces that were bolted to the rod. The accelerometers were suitable up to 10,000 g accelerations.

The signal conditioner/amplifier used in this research was the SCADAS III signal acquisition equipment, model SC 316 front-end system (LMS Difa Instruments Company).

The digital conversion of the data was design oversampling at a rate of 25,600 Hz. This way, the final representation of the data was at a rate of 10,000 Hz.



Figure 1. Instrumented portion of the rod string with the strain gauges and the accelerometers.

## 6 FIELD WORK

Several tests were conducted using an instrumented rod in dynamic probing, in order to calculate the value of  $\eta_4$  and to find out if this value is about

## 6.2 Site

The field work was conducted at Arijá, at the northern part of Burgos, Spain.

The soil consists of a thick layer of sand, being classified as SP in compliance with the USCS classification.

This place was chosen because it consisted of a very homogeneous thick sandy layer. The soil surface was very horizontal and it was easy to access with all the equipment (penetrometer on wheels, bars and vehicle).

In the Figure 2 below it is shown the location of this site. In this figure it is shown the location in Spain and an orthophoto.



Figure 2. Location and orthophoto (SIGPAC).

## 6.3 Experimental results

More than 200 tests were performed at the site explained above.

The value of  $ENTHRU$  was measured by integrating the signals from the instrumented rod.

Then, the value of  $\eta_4$  was calculated by using the Equation 15. This efficiency factor can be expressed in the following form:

$$\eta_4 = \frac{ENTHRU - m_a \cdot g \cdot p}{m_h \cdot g \cdot (h + p)} \quad (19)$$

The values obtained from the field work are shown in Table 1.

As it is revealed in this table, the values show large scattering.

Table 1. Values of the energy efficiency factor obtained from the tests carried out

Series	Number of data tests	Energy efficiency factor ( $\eta_4$ ) (average values)
1	44	0.681
2	39	0.688
3	27	0.718
4	17	0.739
5	37	0.774
6	38	0.771

We can conclude that this value ( $\eta_4$ ) is not the same as the value of  $\eta_1$ . As the value of  $\eta_4$  shows such dispersion, it is assumed the need of measuring the  $ENTHRU$  instead of calculating it from the Equation 15.

## 7 CONCLUSIONS

The way of measuring energy in SPT is different from dynamic probing. The main difference is the skin friction between the drive rods and the soil around them.

It is necessary to use the  $ENTHRU_{cone}$  instead of using the  $ENTHRU$ . The reason is that the energy that really produces penetration is the part of the total energy that effectively reaches the tip of the cone below the rod string.

In order to calculate this  $ENTHRU_{cone}$  it is proposed a new equation (Equation 16). For being able to calculate  $ENTHRU_{cone}$  in a right way, it is necessary to measure the  $ENTHRU$  by means of an instrumented rod. It is not enough accurate to calculate  $ENTHRU$  with Equation 15, because of the wide range or possible values of the energy efficiency factor.

## 8 ACKNOWLEDGMENTS

The writers would like to express their gratitude to the University of Burgos, to the University of Cantabria and to Sondeos del Norte Company.

## 9 REFERENCES

- AENOR. 2006. International Standard UNE-EN ISO 22476-3. Geotechnical investigation and testing. Field testing. Part 3: Standard penetration test. Madrid: AENOR.
- AENOR-CEN (European Committee for Standardization). 2005. International EN ISO 22476-2:2005. Geotechnical investigation and testing. Field testing. Part 2: Dynamic probing (ISO 22476-2:2005). Madrid: AENOR.
- Bergdahl, U. 1979. Development of the dynamic probing test method. Proc. of the 7th European Conference in Soil Mechanic Foundation Engineering in Brighton, England, Volume 2, 201-206. London: British Geotechnical Society.
- Bowles, J.E. 1997. Foundation analysis and design. Madrid: McGraw-Hill.
- Brown, R.E. 1977. Drill rod influence on standard penetration test. Journal of the Geotechnical Engineering Division, Vol. 103, No. GT11, 1332-1336. ASCE.
- Butler, J.J., Caliendo, J.A., Goble, G.G. 1998. Comparison of SPT energy measurement methods. Proc. of the 1st International Conference on Site Characterization, Atlanta, Vol.2, 901-905. Rotterdam: Balkema.
- Card, G.B. & Roche, D.P. 1988. The use of continuous dynamic probing in ground investigation. Proc. of the Geotechnology Conference organized by the Institution of Civil Engineers and held in Birmingham. Penetration Testing in the UK, 119-122. London: Thomas Telford.
- Dahlberg, R. 1975. A comparison between the results from Swedish penetrometers and standard penetration test results in sand". Proc. of the European Symposium On Penetration Testing (ESOPT) in Stockholm. Volume 2:2, 67-68.
- Dahlberg, R. & Bergdahl, U. 1975. Investigations on the Swedish ram-sounding method. Proc. of the European Symposium On Penetration Testing (ESOPT) in Stockholm. Volume 2:2, 93-102.
- Daniel, C.R., Howie, J.A., Jackson, R.S. & Walker, B. 2005. Review of standard penetration test short rod corrections. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 131, No. 4, 489-497. ASCE.
- Daniel, C.R., Howie, J.A., Jackson, R.S. & Walker, B. 2006. Closure to "Review of standard penetration test short rod corrections" by Chris R. Daniel, John A. Howie, R. Scott Jackson, and Brian Walker. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 132, No. 12, 1637-1640. ASCE.
- Danziger, F.A.B., Danziger, B.R. & Cavalcante, E.H. 2006. Discussion of "Review of standard penetration test short rod corrections" by Chris R. Daniel, John A. Howie, R. Scott Jackson, and Brian Walker. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 132, No. 12, 1634-1637. ASCE.
- Décourt, L. 2006. Discussion of "Review of standard penetration test short rod corrections" by Chris R. Daniel, John A. Howie, R. Scott Jackson, and Brian Walker. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 132, No. 12, 1633-1634. ASCE.
- Farrar, J.A. 1998. Summary of Standard penetration test (SPT) energy measurement experience. Proc. of the 1st International Conference on Site Characterization, Atlanta, Vol.2, 919-926. Rotterdam: Balkema.
- Fraile, J. & García, P. 1993. Instrumentación aplicada a la ingeniería. Madrid: Servicio de publicaciones de la E.T.S. Ingenieros de Caminos, Canales y Puertos.
- Housel, W. 1965. Michigan study of pile driving hammers. Journal of the Soil Mechanics and Foundations Division, Vol. 91, No. SM5, 37-64. ASCE.
- Ibáñez, S.J. 2009. Análisis de ensayos de penetración dinámica a través de su rendimiento energético. Thesis presented to the University of Cantabria, at Santander, Spain, in partial fulfillment of the requirements for the degree of Doctor of Philosophy.
- Ibáñez, S.J., Sagaseta, C. & Díez, R. 2006. Relaciones entre resultados de ensayos de penetración dinámica. 7º Simposio Internacional de Estructuras, Geotecnia y Materiales de Construcción. CD de comunicaciones. Santa Clara, Cuba: Universidad Central "Marta Abreu" de Las Villas.
- Johnsen, L.F. & Jagello, J.J. 2007. Discussion of "Energy Efficiency for Standard Penetration Tests" by Edgar Odebrecht, Fernando Schnaid, Marcelo Maia Rocha, and George de Paula Bernardes. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 133, No. 4, 486-487. ASCE.
- Kovacs, W.D. & Salomone, L.A. 1982. SPT hammer energy measurement. Journal of the Geotechnical Engineering Division, Vol. 108, No. GT4, 599-620. ASCE.
- Liao, S.S. & Whitman, R.V. 1986. Overburden correction factors for sand. Journal of the Geotechnical Engineering Division, Vol. 112, No. 3, 373-377. ASCE.
- Matsumoto, T., Sekiguchi, H., Yoshida, H. & Kita, K. 1992. Significance of two-point strain measurement in SPT. Soils and Foundations, Vol. 32, No. 2, 67-82.
- Melzer, K.J. & Smolczyk, U. 1982. Dynamic penetration testing. State-of-the-art report. Proc. of the 2nd European Symposium On Penetration Testing (ESOPT-2) in Amsterdam, 191-202. Rotterdam: Balkema.
- Nixon, I.K. 1982. Standard penetration test. State-of-the-art report. Proc. of the 2nd European Symposium On Penetration Testing (ESOPT-2) in Amsterdam, 3-24. Rotterdam: Balkema.
- Odebrecht, E., Schnaid, F., Maia Rocha, M. & De Paula Bernardes, G. 2005. Energy efficiency for standard penetration tests. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 131, No. 10, 1252-1263. ASCE.
- Odebrecht, E., Schnaid, F., Maia, M. & De Paula, G. 2007. Closure to "Energy Efficiency for Standard Penetration Tests" by Edgar Odebrecht, Fernando Schnaid, Marcelo Maia Rocha, and George de Paula Bernardes. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 133, No. 4, 488-490. ASCE.
- Palacios, A. 1977. The theory and measurement of energy transfer during standard penetration test sampling. Thesis

presented to the University of Florida, at Gainesville, Florida.

- Riggs, C.O. 1986. North American standard penetration test practice: an essay. Proc of In Situ '86 in New York, 949-967. Virginia, USA: ASCE.
- Sánchez Alciturri, J.M. 2001. Apuntes de la Asignatura Reconocimientos Geotécnicos para Obras y Viales. Santander: Servicio de Publicaciones de la Escuela T. S. de Ingenieros de Caminos, C. y P.
- Sancio, R.B. & Bray, J.D. 2005. An assessment of the effect of rod length on SPT energy calculations based on measured field data. Geotechnical Testing Journal, Vol. 28, No. 1, 1-9. ASTM.
- Satoh, K. & Iwasaki, T. 1980. An experimental study of the Swedish automatic ram sounding for several Japanese soils. Proc. of Sounding Symposium, 213-222. Tokio: JSSMFE.
- Schmertmann, J.H. 1970. Static cone to compute static settlement over sand. Journal of Soil Mechanics and Foundation Division, Vol. 96, No. SM3, 101. ASCE.
- Schmertmann, J.H. 1975. Measurement of in-situ shear strength. Proc. ASCE Specialty Conference on In-Situ Measurement of Soil Properties. Raleigh, USA: ASCE.
- Schmertmann, J.H. 1979. Statics of SPT. Journal of the Geotechnical Engineering Division, Vol. 105, No. GT5, Proc. Paper 14573, 655-670. ASCE.
- Schmertmann, J.H. 2007. Discussion of "Energy Efficiency for Standard Penetration Tests" by Edgar Odebrecht, Fernando Schnaid, Marcelo Maia Rocha, and George de Paula Bernardes. Journal of Geotechnical and Geoenvironmental Engineering, Vol. 133, No. 4, 487-488. ASCE.
- Schmertmann, J.H. & Palacios, A. 1979. Energy dynamics of SPT. Journal of the Geotechnical Engineering Division, Vol. 105, No. GT8, Proc. Paper 14769, 909-926. ASCE.
- Soriano, A. 1994. Hincia dinámica y control. Curso sobre pilotajes y cimentaciones especiales, 1-63. Madrid: CEDEX.
- Stefanoff, G., Sanglerat, G., Bergdahl, U. & Melzer, K.J. 1988. Dynamic probing (DP): International reference test procedure. Proc. of the 1st International Symposium On Penetration Testing (ISOPT-1) in Orlando, 53-70. Rotterdam: Balkema.
- Sy, A. & Campanella, R.G. 1991. An alternative method of measuring SPT energy. Proc. of the 2nd International Conference on Recent Advances in Geotechnical Earthquake Engineering and Soil Dynamics in St. Louis, Missouri, USA. 499-505.
- Sy, A. & Campanella, R.G. 1993. Standard penetration test energy measurements using a system based on the personal computer: Discussion. Canadian Geotechnical Journal, Vol. 30, 876-882.